Traffic Operations Review - FINAL Rev.0

Ministry of Transportation and Infrastructure Highway 1 Ladysmith

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1 INTRODUCTION

1.1 Background

R.F. Binnie & Associates Ltd. (Binnie) has been retained by the Ministry of Transportation and Infrastructure (the MOTI) under the current Traffic Engineering General Services Contract No. 880CS0933 to perform a traffic operation review on the Trans Canada Highway (Highway 1) segment through the Town of Ladysmith (the Town). The six intersections reviewed in this report are as follows:

- Highway 1 and Grouhel Road
- Highway 1 and Ludlow Road/1st Avenue
- Highway 1 and Transfer Beach Boulevard/Roberts Street
- Highway 1 and N. Davis Road
- Highway 1 and S. Davis Road
- Highway 1 and Edgelow Road S./Thicke Road

The purpose of this review is to evaluate the performance of these intersections and to provide recommendations for safety and performance improvements. The review will also evaluate the current pedestrian accommodations along this route and provide recommendations for improvement. For this report, Highway 1 is described in the north-south orientation while the minor roads are in the east-west orientation.

The study area is shown in **Figure 1-1**.

1.2 Study Objectives

The objectives of this traffic operations review are outlined as follows:

- Complete traffic signal warrant analysis at select intersections;
- Review existing pedestrian demands across Highway 1 and assess opportunities to improve pedestrian accommodations;
- Review existing operating speeds, vehicle classifications, and collision data on the highway; and
- Review the existing intersection operations at the six study intersections and determine if improvements are required to address existing operational and safety issues.



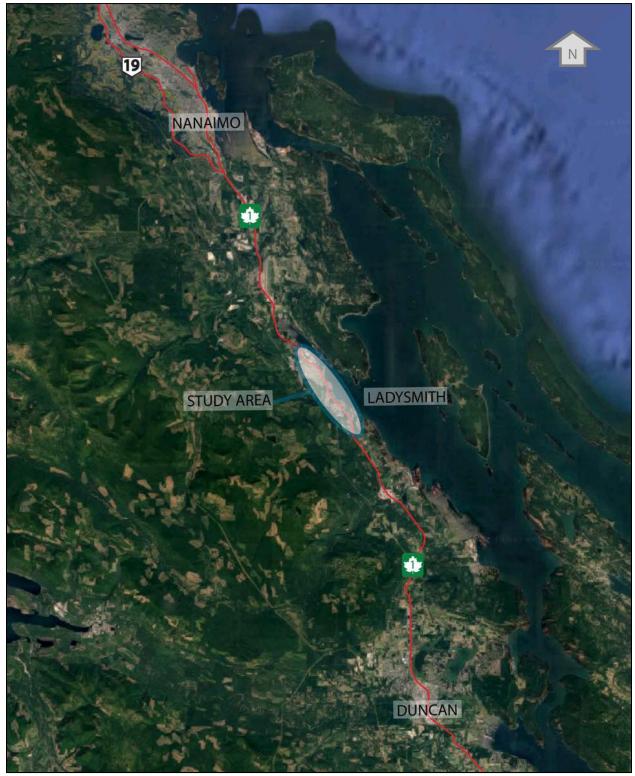


Figure 1-1: Map of Study Area



2 EXISTING CONDITIONS

2.1 Study Road Network

2.1.1 Highway 1

Highway 1 is a major highway that connects the communities on the east side of Vancouver Island and is under the jurisdictions of the MOTI. Within the study area, the highway has a four-lane cross-section, generally runs in the north-south direction through the Town, and has a posted speed of 70 km/h to 90 km/h. A railway track runs adjacent to Highway 1 on the east side.

The highway serves as a major link for the movement of goods and services across Vancouver Island; therefore, trucks make up a high percentage of the highway traffic demands. In addition to truck traffic, Highway 1 also facilitates commuter traffic to and from the nearby City of Nanaimo (Nanaimo).

2.1.2 Grouhel Road

Grouhel Road is a local two-lane roadway that generally runs in the east-west direction and has an assumed speed limit of 50km/h. The existing shoulders are generally grass and narrow, and there are no sidewalks provided. Grouhel Road provides access to a rural residential area.

2.1.3 Ludlow Road/1st Avenue

Ludlow Road/1st Avenue is a collector roadway that runs in the east-west direction. East of Highway 1, Ludlow Road intersects with the existing railroad and provides access to a commercial and industrial area of the Town. To the west of Highway 1, Ludlow Road becomes 1st Avenue, which provides access to the Town and connects residents to Highway 1. Ludlow Road has a four-lane cross-section and 1st Avenue has a two-lane cross-section. Ludlow Road has a posted speed of 40 km/h and 1st Avenue has a posted speed of 30 km/h.

2.1.4 Transfer Beach Boulevard/Roberts Street

Transfer Beach Boulevard/Roberts Street is a two-lane roadway that generally runs in the east-west direction. The roadway has a posted speed of 30 km/h. East of Highway 1, Transfer Beach Boulevard intersects with the existing railroad and connects vehicles to the Town's Transfer Beach Park. To the west of Highway 1, Transfer Beach Boulevard becomes Roberts Street. Roberts Street provides access to the Town and connects residents to Highway 1.

2.1.5 N. Davis Road

N. Davis Road is a two-lane roadway that primarily runs in the east-west direction and has an assumed speed limit of 50km/h. There are existing shoulders and sidewalks provided along the roadway. N. Davis Road provides access to residential and commercial areas.

2.1.6 S. Davis Road

S. Davis Road is a local two-lane roadway that generally runs in the east-west direction and has an assumed speed limit of 50km/h. The existing shoulders are narrow and there are no sidewalks provided.S. Davis Road provides access to a residential area.



2.1.7 Edgelow Road S./Thicke Road

Edgelow Road S./Thicke Road is a two-lane roadway that generally runs in the east-west direction and has an assumed speed limit of 50 km/h. To the west of Highway 1, Edgelow Road S. becomes Thicke Road. The existing shoulders are generally grass and narrow and there are no sidewalks provided. The roadway provides access to a rural residential area.

2.2 Study Intersections

The study road network is an approximately seven km long section of Highway 1 located on Vancouver Island that runs through the Town. It spans from north of the Highway 1 and Grouhel Road intersection to south of the Highway 1 and Edgelow Road S./Thicke Road intersection. The study road network includes the following four signalized intersections:

- Highway 1 and Ludlow Road/1st Avenue intersection
- Highway 1 and Transfer Beach Boulevard/Roberts Street intersection
- Highway 1 and N. Davis Road intersection
- Highway 1 and Edgelow Road S./Thicke Road

The study road network also includes the following two unsignalized intersections:

- Highway 1 and Grouhel Road intersection
- Highway 1 and S. Davis Road intersection

2.2.1 Highway 1 and Grouhel Road Intersection

The intersection of Highway 1 and Grouhel Road is an unsignalized three-legged intersection. The west approach from Grouhel Road is stop-controlled while Highway 1 is free-flowing. The highway has two through lanes in each direction with a northbound left-turn lane and a southbound channelized right-turn lane at the intersection. The eastbound movement has a shared left-turn/right-turn lane, with the right-turn movement channelized. A marked pedestrian crosswalk is provided across the west approach of the intersection.

2.2.2 Highway 1 and Ludlow Road/1st Avenue Intersection

The intersection of Highway 1 and Ludlow Road/1st Avenue is a signalized four-legged intersection. In each direction, the highway has a left-turn lane, two through lanes, and a channelized right-turn lane. The east approach has a left-turn lane, one through lane, and one right-turn lane with an acceleration lane onto the highway. The west approach has a left-turn lane and one shared though/right-turn lane. Vehicles are not permitted to park or stop in the vicinity of the intersection due to the presence of an existing at-grade railway crossing. There are marked pedestrian crosswalks on the east, south, and west approaches of the intersection.

2.2.3 Highway 1 and Transfer Beach Boulevard/Roberts Street Intersection

The intersection of Highway 1 and Transfer Beach Boulevard/Roberts Street is a signalized four-legged intersection. In each direction, the highway has a left-turn lane and two through lanes. The southbound



curb lane is a shared through/right-turn movement, while in the northbound direction, there is a channelized right-turn lane. The east approach has one shared left-turn/through lane and one shared through/right-turn lane, with the right-turn movement channelized. The west approach has one shared left-turn/through lane and one right-turn lane. There are marked pedestrian crosswalks on all four approaches of the intersection.

2.2.4 Highway 1 and N. Davis Road Intersection

The intersection of Highway 1 and N. Davis Road is a signalized four-legged intersection. In each direction, the highway has a left-turn lane, two through lanes, and a channelized right-turn lane. The east approach has a shared left-turn/through lane and a channelized right-turn lane. The west approach has one left-turn lane and one shared through/right-turn lane, with the right-turn movement channelized. Vehicles are not permitted to park or stop in the vicinity of the intersection due to the presence of an existing at-grade railway crossing. There are marked pedestrian crosswalks on the north, east, and west approaches of the intersection.

2.2.5 Highway 1 and S. Davis Road Intersection

The intersection of Highway 1 and S. Davis Road is an unsignalized four-legged intersection. The east and west approaches are stop-controlled while Highway 1 is free-flowing. In each direction, the highway has a left-turn lane, two through lanes, and a channelized right-turn lane. The east and west approaches have one shared left-turn/through/right-turn lane, with the right turn movement channelized.

2.2.6 Highway 1 and Edgelow Raod S./Thicke Road Intersection

The intersection of Highway 1 and Edgelow Road S./Thicke Road is a signalized four-legged intersection. In each direction, the highway has a left-turn lane and two through lanes. The northbound curb lane is a shared through/right-turn movement with the right-turn movement channelized, while in the southbound direction, there is a channelized right-turn lane. The east and west approaches have one shared left-turn/through/right-turn lane. Vehicles are not permitted to park or stop in the vicinity of the intersection. There are marked pedestrian crosswalks on the north, east, and west approaches of the intersection.

2.3 Traffic Volumes

Existing traffic volumes were collected by TransTech Data Services Ltd. (TransTech) on August 9, 2018 for the six study intersections during the weekday AM peak period and PM peak period.

Based on the traffic volume data collected, the AM peak hour of the study corridor was generally found to be from 08:00 to 09:00, with the dominant flow in the southbound direction along Highway 1. The PM peak hour was generally found to be from 16:15 to 17:15, with the dominant flow in the northbound direction.

The study intersections were found to have the following approximate traffic volumes during the AM peak and PM peak hours:

Highway 1 and Grouhel Road: 1,950 vehicles in the AM peak hour and 3,050 vehicles in the PM peak hour



- Highway 1 and Ludlow Road/1st Avenue: 2,100 vehicles in the AM peak hour and 3,200 vehicles in the PM peak hour
- Highway 1 and Transfer Beach Boulevard/Roberts Street: 1,900 vehicles in the AM peak hour and 3,000 vehicles in the PM peak hour
- Highway 1 and N. Davis Road: 1,950 vehicles in the AM peak hour and 3,200 vehicles in the PM peak hour
- Highway 1 and S. Davis Road: 1,550 vehicles in the AM peak hour and 2,450 vehicles in the PM peak hour
- Highway 1 and Edgelow Road S./Thicke Road: 1,500 vehicles in the AM peak hour and 2,350 vehicles in the PM peak hour

The unadjusted turning movement count data are attached to this report in **Appendix A**. The existing traffic volumes for the study intersections are shown below in **Figure 2-1**.

2.4 Pedestrian and Cyclist Accommodation

There are pedestrian sidewalks provided along the west side of Highway 1 between the Ludlow Road/1st Avenue and Methuen Street intersections. Additionally, intersections that connect to the Town's downtown area often have a sidewalk on at least one side of the minor roadway to accommodate pedestrians. There are marked crosswalks at the following intersections within the study area:

- Highway 1 and Grouhel Road (unsignalized) west approach
- Highway 1 and Ludlow Road/1st Avenue (signalized) east, south, and west approaches
- Highway 1 and Kitchener Street (unsignalized) west approach
- Highway 1 and Buller Street (unsignalized) west approach
- Highway 1 and High Street (unsignalized) west approach
- Highway 1 and Gatacre Street (unsignalized) west approach
- Highway 1 and Transfer Beach Boulevard/Roberts Street (signalized) all four approaches
- Highway 1 and Baden Powell Street (unsignalized) west approach
- Highway 1 and N. Davis Road (signalized) north, east, and west approaches
- Highway 1 and Edgelow Road S./Thicke Road (signalized) north, east, and west approaches

Additionally, there is a Highway 1 pedestrian underpass located approximately 200 m south of Methuen Street that provides an alternate route across the highway for pedestrians and cyclists. The underpass provides a connection between the residential area west of the highway and Transfer Beach Park, east of the highway.

Based on information from the Town's website, there are two designated trail routes in the vicinity of the Highway 1 study corridor. The Heritage Walk Trail connects the downtown area to the waterfront via the Highway 1 and Transfer Beach Boulevard/Roberts Street intersection. The Town's section of the Trans Canada Trail is approximately 4.6 km long and crosses Highway 1 at the Ludlow Road/1st Avenue



intersection, the N. Davis Road intersection, and the pedestrian underpass. These trails are commonly used by both pedestrians and cyclists.

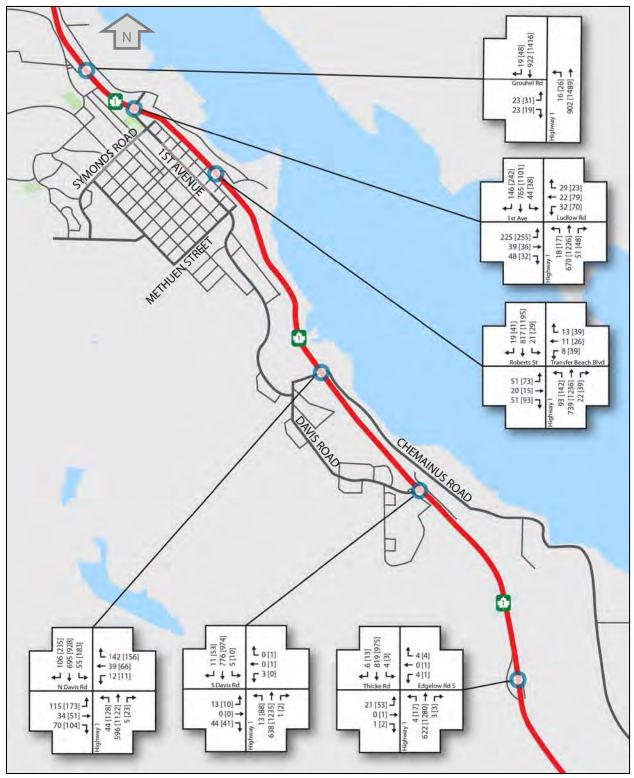


Figure 2-1: Existing Traffic Volumes



3 TRAFFIC OPERATION REVIEW

3.1 Traffic Signal Warrants

Binnie performed MOTI traffic signal warrant analysis at the following intersections:

- Highway 1 and Grouhel Road
- Highway 1 and S. Davis Road

Additionally, Binnie performed a MOTI left-turn signal warrant analysis for the following movement:

Southbound left-turn movement at the Highway 1 and Transfer Beach Boulevard/Roberts Street

The traffic signal warrants assumed an annual growth rate of 2%.

Based on the existing and forecast traffic volumes, a traffic signal is not warranted at the Highway 1 and Grouhel Road intersection unless there is considerable traffic that would detour from the Highway 1 and Ludlow Road/1st Avenue intersection to this intersection should it be signalized. The Highway 1 and S. Davis Road intersection is not warranted for a traffic signal.

The southbound left-turn movement on Highway 1 and Transfer Beach Boulevard/Roberts Street also does not warrant a protected or protected-permissive left-turn traffic signal based on existing and forecast traffic volumes.

The detailed MOTI traffic signal warrant analysis and left-turn signal warrant analysis for the above intersections are summarized in **Appendix B**.

3.2 Pedestrian Crossing Demand

Existing pedestrian volumes were also collected by TransTech at the following locations:

- Highway 1 and Buller Street on Thursday, August 9, 2018 from 8:00 AM to 8:00 PM
- Highway 1 pedestrian underpass near Methuen Street on Sunday, August 12, 2018 from 8:00 AM to 8:00 PM

The Highway 1 and Buller Street intersection is unsignalized and only permits the right-in/right-out (RIRO) and left-in movements. There is a marked crosswalk across the west side street approach; however, there are no marked pedestrian accommodations provided across Highway 1. Based on the collected data, 43 pedestrians were found to cross Highway 1 at Buller Street in a 12-hour period. The peak hour was found to be from 6:00 PM to 7:00 PM with 12 pedestrians crossing Highway 1 during this time.

It is assumed that pedestrians cross at Buller Street to access the Trans Canada Trail, which is easily accessible via a dirt road on the east side of the highway. The nearest Highway 1 pedestrian crossing is approximately 300 m south of Buller Street at the Transfer Beach Boulevard/Roberts Street intersection. It is recommended that a grade-separated pedestrian crossing be provided to accommodate vulnerable



road users near this location. Further discussions between the MOTI and the Town should be undertaken to determine the best location for a pedestrian crossing and what improvements may be necessary along Highway 1 to accommodate it, including the compatibility with the potential waterfront developments planned by the Town.

The Highway 1 pedestrian underpass near Methuen Street provides access between the residential area west of the highway and Transfer Beach Park, Ladysmith Amphitheater, and the waterfront area. Additionally, the pedestrian underpass is part of the Trans Canada Trail. Based on the collected data, 171 pedestrians were found to use the underpass in a 12-hour period. A noticeable increase in use of the pedestrian underpass occurred during the evening. The peak hour was found to be from 5:15 PM to 6:15 PM with 30 pedestrians using the underpass during this time.

3.3 Highway 1 Operating Speeds

Existing two-way vehicle operating speed data were collected by TransTech along Highway 1 from August 9, 2018 to August 15, 2018, for 24 hours per day, at the following locations along Highway 1:

- North of Grouhel Road
- Between Ludlow Road/1st Avenue and Transfer Beach Boulevard/Roberts Street
- 700 m south of Transfer Beach Boulevard/Roberts Street
- Between N. Davis Road and S. Davis Road
- 400 m south of S. Davis Road
- 400 m south of Edgelow Road S./Thicke Road

Currently, Highway 1 southbound traffic transitions from 90 km/h to 70 km/h approximately two km north of the Grouhel Road intersection, while the northbound traffic transitions from 70 km/h to 90 km/h approximately 100 m north of the intersection. North of Grouhel Road, the existing average vehicle speed on Highway 1 was found to be between 95 km/h and 100 km/h for both northbound and southbound traffic, while the 85th percentile vehicle speed was found to be between 105 km/h and 110 km/h for both directions.

Between Ludlow Road/1st Avenue and Transfer Beach Boulevard/Roberts Street, the Highway 1 posted speed limit is 70 km/h. The existing average vehicle speed was found to be between 65 km/h and 70 km/h for southbound traffic and between 75 km/h and 80 km/h for northbound traffic. The 85th percentile vehicle speed was found to be between 75 km/h and 80 km/h for southbound traffic and between 85 km/h and 90 km/h for northbound traffic.

Approximately 700 m south of Transfer Beach Boulevard/Roberts Street near Gifford Road, Highway 1 southbound traffic transitions from 70 km/h to 90 km/h, while the northbound traffic transitions from 90 km/h to 70 km/h. The existing average vehicle speed was found to be between 80 km/h and 85 km/h for southbound traffic and approximately 70 km/h for northbound traffic. The 85th percentile vehicle speed was found to be between 90 km/h and 100 km/h for southbound traffic and approximately 85 km/h for northbound traffic.



Between N. Davis Road and S. Davis Road, the existing posted speed limit along Highway 1 is 90 km/h. The existing average vehicle speed was found to be between 95 km/h and 100 km/h for both northbound traffic and southbound traffic. The 85th percentile vehicle speed was found to be approximately 105 km/h for both directions.

400 m south of S. Davis Road, the existing posted speed limit along Highway 1 is 90 km/h. The existing average vehicle speed was found to be approximately 100 km/h for both northbound traffic and southbound traffic. The 85th percentile vehicle speed was found to be approximately 110 km/h for southbound traffic and between 110 km/h and 115 km/h for northbound traffic.

400 m south of Edgelow Road S./Thicke Road, the existing posted speed limit along Highway 1 is 90 km/h. The existing average vehicle speed was found to be between 95 km/h and 100 km/h for both northbound traffic and southbound traffic. The 85th percentile vehicle speed was found to be between 105 km/h and 110 km/h for southbound traffic and between 110 km/h and 115 km/h for northbound traffic. Currently, the nearest northbound posted speed limit sign is approximately 2.6 km south of the intersection.

Along the entire study corridor, the existing vehicle operating speed along Highway 1 is approximately 10 km/h to 25 km/h higher than the posted speed limit, as can be seen in **Table 3-1**.

Higwhay 1 Location /	North of	Between Ludlow	Near Gifford	Between N.	400m south	400m south
Posted Speed	70	70	70	90	90	90
NB Average	95-100	75-80	70	95-100	100	95-100
NB 85th Percentile	105-110	85-90	85	105	110-115	110-115
SB Average	95-100	65-70	80-85	95-100	100	95-100
SB 85th Percentile	105-110	75-80	90-100	105	100	105-110

Table 3-1: Summary of Existing Speed Classification

Based on the MOTI's *Technical Circular T-04/14 Corridor Speed Limit Procedure and Speed Limit Approvals*, dated June 23, 2014, the posted speed limit on a MOTI highway correlates, in part, with the 85th percentile vehicle operating speed on a highway segment in ideal conditions. Historically, when the posted speed limit is inconsistent with the speed that drivers are comfortable travelling at through a highway segment, issues such as driver frustration, excessive speeding, and unsafe driving manoeuvres can arise.

Given that in each segment of the study corridor the 85th percentile vehicle operating speed is higher than the posted speed limit, any further speed limit reduction is expected to have low compliance by drivers and it may potentially exacerbate any current safety concerns, e.g., significant speed differentials in operating speed that could result in more serious collisions; therefore, it is recommended that the current posted speed limits within the study area of Highway 1 be maintained and perform periodic enforcement to ensure the operating speed is consistent with the posted speed.

3.4 Highway 1 Vehicle Classification

Existing two-way vehicle classification data were collected by TransTech along Highway 1 from August 9, 2018 to August 15, 2018, for 24 hours per day, at the following locations along Highway 1:



- North of Grouhel Road
- Between Ludlow Road/1st Avenue and Transfer Beach Boulevard/Roberts Street
- 700 m south of Transfer Beach Boulevard/Roberts Street
- Between N. Davis Road and S. Davis Road
- 400 m south of S. Davis Road
- 400 m south of Edgelow Road S./Thicke Road

The vehicle survey classifies the following vehicle types as passenger vehicles:

- Motorcycles
- Passenger cars, including those with recreational trailers
- Two-axle pickup trucks, vans, and RVs, including those with recreational trailers

The vehicle survey classifies the following vehicle types as heavy trucks:

- Buses
- Two-axle, six-wheel vehicles
- Three+ axle vehicles as a single or double unit

Based on the data collected, two-way traffic along Highway 1 within the study corridor is approximately 83% passenger vehicles and 17% heavy trucks. The vehicle classification data is summarized in **Table 3-2** and the unadjusted survey results are attached in **Appendix C**.

Highway 1 Location	Pas	Passenger Vehicles			Trucks			
Highway 1 Location	NB	SB	Two-Way%	NB	SB	Two-Way%		
North of Grouhel Rd	84.4%	83.3%	83.9%	15.6%	16.7%	16.2%		
Between Ludlow Rd / 1st Ave and	84.1%	82.2%	83.2%	15.9%	17.8%	16.9%		
Near Gifford Rd	81.4%	82.8%	82.1%	18.6%	17.2%	17.9%		
Between N. Davis Rd and S. Davis Rd	82.2%	82.9%	82.6%	17.8%	17.1%	17.5%		
400m south of S. Davis Rd	84.0%	83.4%	83.7%	16.0%	16.6%	16.3%		
400m south of Thicke Rd	83.8%	83.3%	83.6%	16.2%	16.7%	16.5%		
Average	83.3%	83.0%	83.2%	16.7%	17.0%	16.9%		

Table 3-2: Summary of Traffic Classification

The MOTI permanent count station P-12-3NS is located approximately 10 km north of the study area. In 2017, two-way traffic on Highway 1 was approximately 90% passenger vehicles and 10% heavy trucks, which supports the vehicle classification survey results.

3.5 Highway 1 Collision Data

Existing collision data at the study intersections were provided by the MOTI and the Insurance Corporation of British Columbia (ICBC). ICBC data were recorded from 2011 to 2015 and are based on driver reported collisions. The MOTI data were recorded from 2012 to 2016 and are based on police reported collisions. The collision summary reports are attached in **Appendix D**.



3.5.1 Highway 1 and Grouhel Road Intersection

Based on ICBC data, the Highway 1 and Grouhel Road intersection had 12 property damage only (PDO) incidents and 11 collisions with injuries caused to drivers or passengers during the five-year period. No fatalities were reported. The histogram in **Figure 3-1** summarizes the collision data provided by ICBC between 2011 and 2015.

Based on MOTI data, police responded to four collisions at this intersection between 2012 and 2016. Three of the collisions involved a vehicle making a 90° left-turn movement at the intersection, while one collision involved a vehicle leaving the travel lane to the right-hand side. **Figure 3-2** summarizes the type of collisions at the intersection of Highway 1 and Grouhel Road.

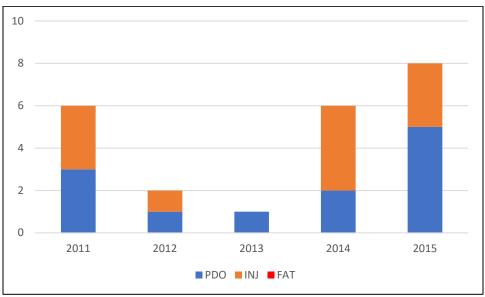


Figure 3-1: Collision Severity at Highway 1 and Grouhel Road



Figure 3-2: Collision Types at Highway 1 and Grouhel Road



3.5.2 Highway 1 and Ludlow Road/1st Avenue intersection

Based on ICBC data, the Highway 1 and Ludlow Road/1st Avenue intersection had 19 PDO incidents and ten collisions with injuries caused to drivers or passengers during the five-year period. No fatalities were reported. The histogram in **Figure 3-3** summarizes the collision data provided by ICBC between 2011 and 2015.

Based on MOTI data, police responded to eight collisions at this intersection between 2012 and 2016. Four of the incidents involved rear-end collisions, with two occurring in the northbound direction and two occurring in the southbound direction. Three collisions involved vehicles making the left-turn movement from the highway onto the side street, and one incident involved a head-on collision. **Figure 3-4** summarizes the type of collisions at the intersection of Highway 1 and Ludlow Road/1st Avenue.

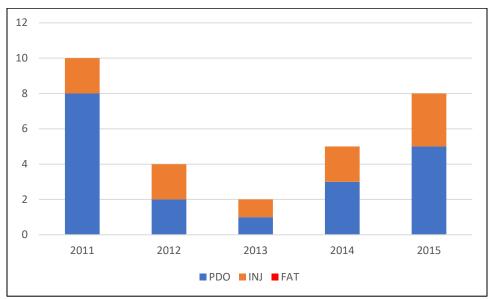


Figure 3-3: Collision Severity at Highway 1 and Ludlow Road/1st Avenue



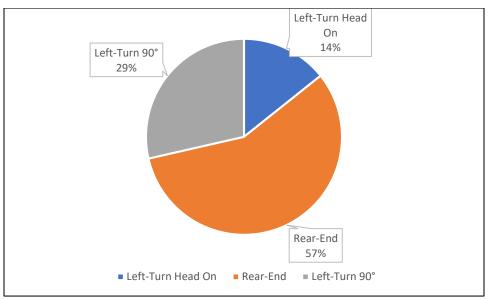


Figure 3-4: Collision Types at Highway 1 and Ludlow Road/1st Avenue

3.5.3 Highway 1 and Transfer Beach Boulevard/Roberts Street Intersection

Based on ICBC data, the Highway 1 and Transfer Beach Boulevard/Roberts Street intersection had 12 PDO incidents and 14 collisions with injuries caused to drivers or passengers during the five-year period. No fatalities were reported. The histogram in **Figure 3-5** summarizes the collision data provided by ICBC between 2011 and 2015.

Based on MOTI data, police responded to six collisions at this intersection between 2012 and 2016. Two of the incidents involved rear-end collisions in the northbound direction. Two collisions involved vehicles making the left-turn movement and one incident was a single vehicle leaving the travel lane. Lastly, one collision was reported as "other" and no further details were provided. **Figure 3-6** summarizes the type of collisions at the intersection of Highway 1 and Transfer Beach Boulevard/ Roberts Street.



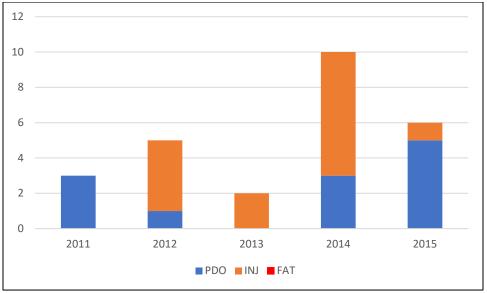


Figure 3-5: Collision Severity at Highway 1 and Transfer Beach Blvd./Roberts St.

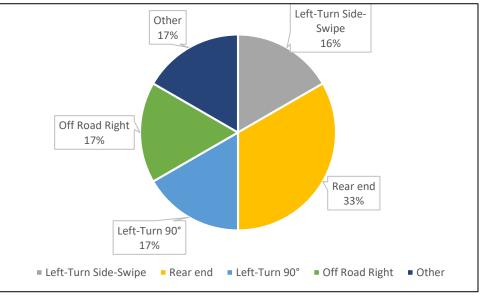


Figure 3-6: Collision Types at Highway 1 and Transfer Beach Blvd./Roberts St.

3.5.4 Highway 1 and N. Davis Road Intersection

Based on ICBC data, the Highway 1 and N. Davis Road intersection had 29 PDO incidents and 20 collisions with injuries caused to drivers or passengers during the five-year period. No fatalities were reported. The histogram in **Figure 3-7** summarizes the collision data provided by ICBC between 2011 and 2015.

Based on MOTI data, police responded to ten collisions at this intersection between 2012 and 2016. Seven of the incidents involved rear-end collisions with one in the southbound direction, five in the northbound direction, and one unspecified. One collision was a side-swipe incident during an overtaking manoeuvre and another incident was a single vehicle leaving the travel lane during



inclement weather conditions. Lastly, one collision involved hitting a wild animal. **Figure 3-8** summarizes the type of collisions at the intersection of Highway 1 and N. Davis Road.

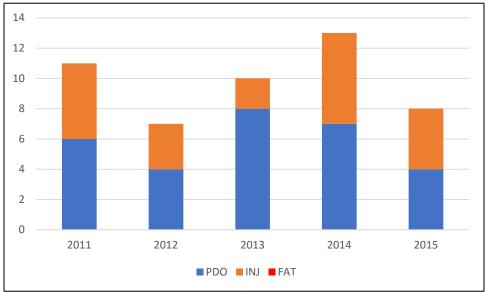


Figure 3-7: Collision Severity at Highway 1 and N. Davis Road

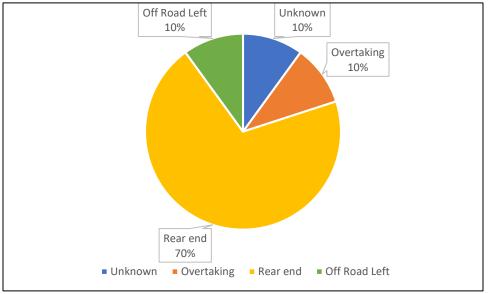


Figure 3-8: Collision Types at Highway 1 and N. Davis Road

3.5.5 Highway 1 and S. Davis Road Intersection

Based on ICBC data, there were no collisions at the Highway 1 and S. Davis Road between 2011 and 2015. Based on MOTI data, police responded to one rear-end collisions at this intersection between 2012 and 2016.

It is noted that in 2017, a collision that resulted in injury caused to drivers and passengers occurred at this intersection between an eastbound vehicle and a northbound vehicle. Although the primary collision type is unknown, both vehicles left the roadway due to the collision.



3.5.6 Highway 1 and Edgelow Road S./Thicke Road Intersection

Based on ICBC data, the Highway 1 and Edgelow Road S./Thicke Road intersection had four PDO incidents and six collisions with injuries caused to drivers or passengers during the five-year period. No fatalities were reported. The histogram in **Figure 3-9** summarizes the collision data provided by ICBC between 2011 and 2015.

Based on MOTI data, police responded to nine collisions at this intersection between 2012 and 2016. Six of the incidents involved rear-end collisions in the northbound direction. One collision involved a vehicle making a 90° left-turn movement at the intersection and one incident was a single vehicle leaving the travel lane. Lastly, one collision involved hitting a wild animal. **Figure 3-10** summarizes the type of collisions at the intersection of Highway 1 and Ludlow Road/1st Avenue.

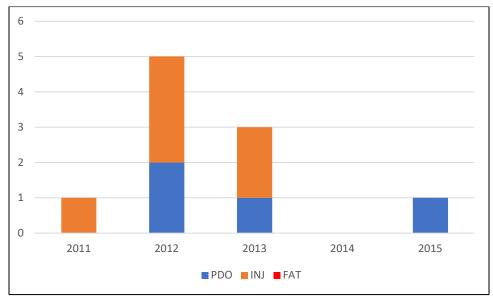


Figure 3-9: Collision Severity at Highway 1 and Edgelow Road S./Thicke Road



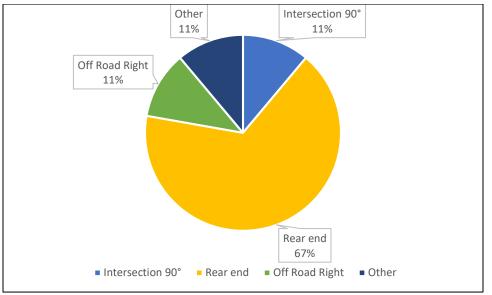


Figure 3-10: Collision Types at Highway 1 and Edgelow Road S./Thicke Road

Traffic Operation Analysis 3.6

3.6.1 Methodology

The traffic operation analysis in this report was performed using the Synchro 9 software suite, which is generally based on the Highway Capacity Manual (HCM) methodologies. The existing traffic operations were evaluated to estimate the volume to capacity (v/c) ratio, delay, level-of-service (LOS), and 95th percentile queue length at each of the study intersections.

When reviewing the traffic analysis results, a v/c ratio at or above 1.00 indicates that traffic volumes exceed the intersection capacity. Delay, in terms of seconds, represents the wait time experienced by a driver on the approach to the intersection. LOS is a grading system on intersection operation based on the calculated delay as per the criteria shown in Table 3-3 for a signalized intersection and in Table 3-4 for an unsignalized intersection. LOS A means that the intersection experiences little to no delay whereas a LOS F indicates significant delay is present.

Table 3-3: HCM LOS Criteria for Signalized Intersection			
Level of Service	Average Control Delay (s/veh)		
Â	0 – 10		
В	> 10 – 20		
С	> 20 – 35		
D	> 35 – 55		
E	> 55 - 80		
F	> 80		

1.11



Level of Service	Average Control Delay (s/veh)
А	0 – 10
В	> 10 – 15
С	> 15 – 25
D	> 25 – 35
E	> 35 – 50
F	> 50

Table 3-4: HCM LOS Criteria for Unsignalized Intersection

The target intersection operation thresholds for this study were assumed to be as follows:

- LOS D or better for the overall intersection and individual turning movements;
- Delay less than 55 seconds (signalized) or 35 seconds (unsignalized) at an intersection; and
- v/c ratio of 0.85 or lower for the overall intersection and individual turning movements.

The detailed traffic analysis results output from Synchro are provided in **Appendix E**. The existing signal timing plans for the study road network were provided by the MOTI and were used in the following analysis.

3.6.2 Existing AM Peak Hour

Based on the Synchro analysis, all existing intersections within the study area were found to be operating overall at LOS B or better during the AM peak hour.

3.6.3 Existing PM Peak Hour

Based on the Synchro analysis, all existing intersections within the study area were found to be operating overall at LOS C or better during the PM peak hour; however, some intersections have individual turning movements that operate above the study thresholds.

At the Highway 1 and Grouhel Road intersection, the eastbound shared left-turn/right-turn movement was found to be operating at LOS F with a v/c ratio of 1.38.

At the Highway 1 and Ludlow Road/ 1^{st} Avenue intersection, the eastbound left-turn movement was found to be operating at LOS F with a v/c ratio of 1.04.

At the Highway 1 and N. Davis Road intersection, the northbound and southbound left-turn movements were both found to be operating at LOS E with v/c ratios of 0.65 and 0.72, respectively. Additionally, the eastbound left-turn movement and westbound shared left-turn/through movement were both found to be operating at LOS E with v/c ratio of 0.78 and 0.53, respectively.

At the Highway 1 and S. Davis Road intersection, the eastbound shared left-turn/through/right-turn movement and the westbound shared left-turn/through movement were both found to be operating at LOS F with v/c ratios of 0.44 and 0.05, respectively.

The existing traffic operations for the AM peak and PM peak are summarized in Table 3-5.



Table 3-5: Existing Traffic Operations

Table 3-5: Existing Tr	Turning		AM Pea	ak Hour			PM Pea	ak Hour	
Intersection	Movement	LOS			95% Q (m)	LOS	Delay (s)	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	95% Q (m
	EBL/R	D	33.5	0.28	8.5	F	432.5	1.38	42.4
	NBL	В	10.4	0.02	0.6	В	14.0	0.07	1.6
Hwy1/GrouhelRd	NBT	A	-	0.29	-	A	-	0.48	-
(Unsignalized)	SBT	A	-	0.29	-	A	-	0.45	-
	SBR	A	-	0.01	-	A	-	0.03	-
	Int. LOS		I	A			I	4	
	EBL	D	36.9	0.69	#71.6	F	101.9	1.04	#102.0
	EBT/R	В	13.7	0.20	16.2	В	19.8	0.19	16.8
	WBL	С	23.3	0.11	11.3	С	33.5	0.29	23.6
	WBT	С	22.6	0.05	8.5	С	31.6	0.23	25.3
	WBR	A	-	0.02	-	A	-	0.02	-
Hwy1&1st	NBL	В	13.4	0.08	5.3	В	11.2	0.08	4.8
Ave/Ludlow Rd	NBT	В	15.3	0.50	49.4	В	16.4	0.70	106.6
(Signalized)	NBR	Α	0.6	0.08	1.1	Α	0.1	0.06	0.3
	SBL	A	6.5	0.12	5.6	A	6.1	0.16	4.8
	SBT	Α	9.4	0.45	36.7	A	8.9	0.54	57.8
	SBR	A	1.7	0.18	5.8	A	1.3	0.24	6.7
	Int. LOS	İ		B	•			В	******
	EBL/T	С	32.7	0.35	23.5	D	44.5	0.52	29.8
	EBR	A	4.2	0.17	4.6	В	10.5	0.33	12.9
	WBL/T	С	27.6	0.05	4.5	С	33.5	0.18	11.3
	WBR	A	-	0.01	-	A	-	0.03	-
Hwy1&RobertsSt	NBL	Α	5.2	0.21	9.0	В	13.8	0.49	24.1
(Signalized)	NBT	A	4.8	0.31	31.2	A	7.0	0.53	71.2
	NBR	A	0.7	0.02	1.1	A	1.6	0.04	2.8
	SBL	В	12.0	0.06	6.0	В	14.6	0.16	8.9
	SBT/R	В	13.5	0.46	67.4	В	19.1	0.74	125.0
	Int. LOS			B			ĺ	3	******
	EBL	С	27.7	0.40	33.5	E	61.5	0.78	#67.2
	EBT/R	В	11.9	0.24	18.1	С	24.0	0.43	38.0
	WBL/T	D	40.2	0.30	21.3	E	61.8	0.53	35.9
	WBR	A	0.2	0.11	-	A	0.2	0.12	-
Ihurd & N.D. de Dil	NBL	D	40.0	0.26	19.3	E	62.0	0.65	53.3
Hwy 1 & N Davis Rd	NBT	С	20.9	0.50	62.8	D	35.6	0.84	#202.9
(Signalized)	NBR	A	-	0.01	-	A	0.1	0.04	-
	SBL	D	39.9	0.31	22.5	E	59.2	0.72	68.9
	SBT	В	19.5	0.53	73.7	С	25.1	0.64	127,7
	SBR	Α	4.3	0.16	9.6	А	3.5	0.31	14.5
	Int. LOS			B				C	
	EBL/T/R	С	18.5	0.19	5.2	F	51.7	0.43	14.2
	WBL/T	D	31.8	0.02	0.5	F	194.8	0.05	1.1
	WBR	-	-	0.00	-	В	14.0	0.00	-
	NBL	Α	9.6	0.02	0.4	В	11.5	0.15	3.9
Hwy1&DavisRd	NBT	A	-	0.20	-	А	-	0.39	-
(Unsignalized)	NBR	Α	-	0.00	-	А	-	0.00	-
	SBL	A	9.0	0.01	0.1	В	12.2	0.02	0.5
	SBT	Α	-	0.25	-	Α	-	0.31	-
	SBR	A	-	0.01	-	А	-	0.03	-
	Int. LOS		1	A				4	
	EBL/T/R	А	4.1	0.07	2.9	С	29.4	0.28	17.7
Ibuu 4 9 Thiolog	WBL/T/R	А	0.1	0.02	-	В	20.0	0.02	3.3
	NBL	Α	2.5	0.01	0.9	А	4.2	0.04	2.6
Hwy 1 & Thicke	NBT/R	A	1.7	0.21	19.1	А	4.9	0.47	59.3
Rd/Edgelow Rd (Signalized)	SBL	Α	2.5	0.01	0.9	Α	4.0	0.01	0.9
(Signalized)	SBT	Α	1.9	0.27	26.3	Α	4.1	0.37	40.5
	SBR	A	0.2	0.00	0.2	A	0.5	0.01	0.6



4 PROPOSED IMPROVEMENTS

4.1 Signal Timing Improvements

4.1.1 Highway 1 and Ludlow Road/1st Avenue Intersection

The Highway 1 and Ludlow Road/1st Avenue intersection is currently operating at a cycle length of 96.3 seconds. By increasing the cycle length to 100 seconds and optimizing the length of green time provided to each phase, the intersection's maximum v/c ratio is expected to decrease to be within study thresholds and the eastbound left-turn movement is expected to operate at LOS D.

As the intersection is currently over capacity, signal timing improvements can only increase the capacity by so much. As the area continues to develop and grow in population and employment opportunities in the future, geometric improvements to the intersection will be required to achieve additional capacity.

4.1.2 Highway 1 and Transfer Beach Boulevard/Roberts Street intersection

Redistributing the green time at this signal provides negligible changes to intersection performance in the AM peak hour. In the PM peak hour, redistributing the green time results in a minor delay decrease for the Highway 1 movement and a minor delay increases for the eastbound and westbound movements. Therefore, the existing signal timing may be maintained at this intersection.

4.1.3 Highway 1 and N. Davis Road

The Highway 1 and N. Davis Road intersection is currently operating at a cycle length of 140.5 seconds to facilitate more Highway 1 green time and to minimize the lost time. Although the eastbound left-turn movement was found to be operating at LOS E in the PM peak hour, providing more green time to this movement did not provide a noticeable improvement to the intersection as the delay is likely a result of the long Highway 1 green time. To increase the capacity of the intersection and improve the performance of the eastbound left-turn movement, it is likely that geometric improvements will be required.

4.1.4 Highway 1 and Edgelow Road S./Thick Road

As the Highway 1 and Edgelow Road S./Thicke Road intersection is operating at an overall LOS A for both AM and PM peak, and no individual movements are operating below LOS C, no signal timing improvements are recommended for this intersection.

4.2 Intersection Operation Improvements

4.2.1 Highway 1 and Grouhel Road Intersection

At the Highway 1 and Grouhel Road intersection, the shared eastbound left-turn/right-turn movement currently operates at LOS F during the PM peak as vehicles have difficulty finding suitable gaps on Highway 1 to make the left-out movement. As this intersection does not warrant a traffic signal, as discussed in **Section 3.1**, an alternative improvement option was considered.



Restricting the left-out movement at the Highway 1 and Grouhel Road intersection is expected to improve the eastbound movement to LOS C in the PM peak hour. This restriction would require traffic to detour to the Highway 1 and Ludlow Road/1st Avenue intersection via Christie Road, 3rd Avenue, and Symonds Street to make the eastbound left-turn movement. Alternatively, a protected-T intersection may be considered as well subject to detailed analysis regarding the platooning effects on its performance due to the traffic signal nearby.

4.2.2 Highway 1 and Ludlow Road/1st Avenue Intersection

As the Highway 1 and Ludlow Road/1st Avenue intersection eastbound left-turn movement is currently operating at LOS F, the addition of more vehicles making this movement is expected to further increase the delay; however, by implementing the signal timing improvements discussed in **Section 4.1**, additional capacity can be accommodated by this movement and the expected PM peak delay may improve to LOS D.

Alternatively, based on information provided by the Town, a roundabout has been proposed at the existing Rocky Creek Road and Ludlow Road T-intersection. Should this proposed reconfiguration be constructed, the Grouhel Road eastbound vehicles would be able to access Highway 1 northbound by making the right-out movement onto Highway 1 southbound, making the left-turn movement onto Ludlow Road, and using the roundabout as a turnaround facility. Given the existing low southbound left-turn volume at the Highway 1 and Ludlow Road/1st Avenue intersection, which operates as a protected-permissive movement, it is expected that the intersection will be able to accommodate the additional vehicles without any significant impacts.

4.2.3 Highway 1 and S. Davis Road Intersection

The Highway 1 and S. Davis Road intersection was found to experience significant delays in the eastbound shared left-turn/through/right-turn movement and the westbound shared left-turn/ through movement in the PM peak hour. As this intersection does not warrant a traffic signal, as discussed in **Section 3.1**, it is proposed that the Highway 1 and S. Davis Road intersection be restricted to the right-in/right-out (RIRO) and left-in movements only. It is recommended that the Highway 1 left-in movements are maintained as they are currently operating within the study thresholds.

Under the proposed laning configuration, vehicles making the existing S. Davis Road eastbound leftout movement will be required to detour to the signalized N. Davis Road intersection to access Highway 1 northbound. Existing eastbound through vehicles will also be required to use this signalized intersection to access the west side of S. Davis Road via the Highway 1 southbound left-in movement. Similarly, the current westbound left-out movement will be required to use the N. Davis Road intersection and Davis Road as a turnaround facility to access Highway 1 southbound via the S. Davis Road eastbound right-out movement. Lastly, the existing westbound through movement will be able to access the east side of S. Davis Road via the signalized N. Davis Road intersection as well.

Given the low existing traffic volumes making the left-turn and through movements from S. Davis Road, it is expected that the N. Davis Road intersection will be able to accommodate the additional vehicles without any significant impact. It is noted that although the northbound left-turn movement at the



Highway 1 and N. Davis Road intersection is operating at LOS E, the movement has a v/c ratio of 0.65. This indicates the delay is due to the long signal cycle length and that the movement has reserve capacity to accommodate additional vehicles.

This option is preferred over a speed limit reduction along Highway 1 as it is assumed a lower speed will receive low compliance by drivers.

4.3 Pedestrian Accommodations

4.3.1 Highway 1 and Buller Street

At the unsignalized Highway 1 and Buller Street intersection, there are no marked pedestrian accommodations provided across Highway 1. Based on the collected data, 43 pedestrians were found to cross Highway 1 at Buller Street in a 12-hour period. The peak hour was found to be from 6:00 PM to 7:00 PM with 12 pedestrians crossing Highway 1 during this time.

To improve pedestrian safety at the Highway 1 and Buller Street intersection, it is recommended that pedestrians be discouraged from crossing the highway at this location as there are no marked facilities. In the short-term, the following strategy is recommended:

Install median fencing to direct pedestrians towards the nearest marked intersection.

In the long-term, the following additional strategy is recommended to improve pedestrian safety:

• Coordinate with the Town to construct a grade-separated pedestrian crossing near this location.

4.3.2 Highway 1 Pedestrian Underpass

The Highway 1 pedestrian underpass near Methuen Street provides access between the residential area west of the highway and Transfer Beach Park, Ladysmith Amphitheater, and the waterfront area. Additionally, the pedestrian underpass is part of the Trans Canada Trail. Based on the collected data, 171 pedestrians were found to use the underpass in a 12-hour period. A noticeable increase in use of the pedestrian underpass occurred in the evening. The peak hour was found to be from 5:15 PM to 6:15 PM with 30 pedestrians using the underpass during this time.

The Highway 1 pedestrian underpass near Methuen Street is well utilized by pedestrians and cyclists as part of the Trans Canada Trail. To enhance user's experience along the trail, the MOTI may work with the Town to provide future public space improvements at the underpass.

In the long-term, the following additional strategy is recommended to enhance user experience:

• Coordinate with the Town to construct additional multi-use pathways on the east side of the highway that connect to existing trails for recreational use.

4.4 Safety Improvements

The following safety improvements are proposed for the six study intersections:



- Additional speed limit signage for southbound vehicles on Highway 1 upstream of Grouhel Road to alert drivers of the speed transition from 90 km/h to 70 km/h and to encourage drivers to slow down in more urban areas. The slower speeds will also benefit vehicles making the leftin and left-out movements at Grouhel Road.
- At the intersection of Highway 1 and Ludlow Road/1st Avenue, the Town has noted that some driver confusion has been observed for the southbound left-turn movement. Due to the wide chevron gore between the westbound through lane and left-turn lane, drivers confuse the location of the receiving lane. Providing intersection guiding lines for the southbound left-turn movement may improve the guidance for drivers making this turn.
- For the Highway 1 and Transfer Beach Boulevard/Roberts Street intersection, it is recommended that the signal timing sheet's Intersection Flash be updated to Red for all approaches. This will improve the safety of the egress traffic from the side roads in the event of a power outage or a signal controller fault.
- At the intersection of Highway 1 and N. Davis Road, rear-end collisions accounted for 70% of all collisions that occurred at this location based on the MOTI data, especially in the northbound direction. Until capacity improvements are provided at this intersection, active congestion ahead warning sign could be considered for the northbound traffic to supplement the advance warning flashers that are currently in place.
- Additional speed limit signage for Highway 1 northbound vehicles on the north side of the N.
 Davis Road intersection to alert drivers of the speed transition from 90 km/h to 70 km/h and to encourage drivers to slow down in more urban areas.
- Install No Right Turn signage on the west approach of the Highway 1 and Edgelow Road S./Thicke Road intersection. There is a yield-controlled eastbound right-turn lane onto Highway 1 southbound approximately 120 m south of the intersection to facilitate the turnaround movement for Highway 1 northbound vehicles. The available sightline for eastbound right-turn vehicles is better at the turnaround location than the upstream intersection due to the existing vertical grade of Highway 1 in the southbound direction.



5 CONCLUSION AND RECOMMENDATIONS

5.1 Conclusion

Binnie has been retained by the MOTI to perform a traffic operation review on Highway 1 through the Town. The purpose of this review is to evaluate the performance of six intersections and to provide recommendations for safety and performance improvements. The study intersections are:

- Highway 1 and Grouhel Road
- Highway 1 and Ludlow Road/1st Avenue
- Highway 1 and Transfer Beach Boulevard/Roberts Street
- Highway 1 and N. Davis Road
- Highway 1 and S. Davis Road
- Highway 1 and Edgelow Road S./Thicke Road

The performance of the above intersections was analyzed based on the existing traffic volumes, operating speed, vehicle classification, and collision data. A summary of the study findings are as follows:

- All intersections within the study area were found to be operating overall at LOS B or better during the AM peak hour and at LOS C or better during the PM peak hour
- Traffic signals are not warranted at the Highway 1 and Grouhel Road intersection or the Highway 1 and S. Davis Road intersection. The southbound left-turn movement on Highway 1 and Transfer Beach Boulevard/ Roberts Street also does not warrant a protected or protectedpermissive left-turn traffic signal
- Traffic along Highway 1 generally travels above the posted speed limit through the study area by approximately 10 km/h to 25 km/h
- Two-way traffic along Highway 1 within the study corridor is approximately 83% passenger vehicles and 17% heavy trucks
- Based on ICBC data, 29 PDO incidents and 20 collisions with injury occurred at the intersection of Highway 1 and N. Davis Road between 2011 and 2015
- Significant number of pedestrians cross Highway 1 near Buller Street with seven pedestrians accounted for during the peak

5.2 **Recommendations**

The following recommendations are based on the analysis findings outlined in this report.

Short-Term

• Restrict the eastbound left-turn movement at the Highway 1 and Grouhel Road intersection to improve the traffic operations and safety at the unsignalized intersection



- Additional posted speed limit signage upstream of Grouhel Road for Highway 1 southbound traffic
- Implement an updated signal timing sheet at the Highway 1 and Ludlow Road/1st Avenue intersection that optimizes the green time provided for each phase
- Apply intersection guiding lines for the southbound left-turn movement at the intersection of Highway 1 and Ludlow Road/1st Avenue
- Consider installing an active congestion ahead warning sign for the northbound traffic approaching the Highway 1 and N. Davis Road intersection.
- Additional posted speed limit signage downstream of N. Davis Road for Highway 1 northbound traffic
- Restrict the unsignalized intersection of Highway 1 and S. Davis to RIRO and left-in movements only
- On Highway 1 near Buller Street, direct pedestrians to intersections with marked crosswalks by installing median fences

Mid-Term/Long-Term

- Geometric design changes at the intersection of Highway 1 and Ludlow Road/1st Avenue and the intersection of Highway 1 and N. Davis Road for noticeable traffic operations improvements
- Consider grade-separated pedestrian crossing facility near Highway 1 at Buller Street that is compatible with the planned waterfront developments in the Town.



APPENDIX A

EXISTING TURNING MOVEMENT COUNT DATA





Major Route: Minor Route: Municipality: Filename: Location #:	Highway 1 Grouhel Road Ladysmith 1-Highway 1 @ Grouhel Rd-Aug 1	9, 2018.xlsx
Date:	August 9, 2018	
Day-of-week:	Thursday	
East/West Route:	Grouhel Road	
Intersection Type:	3-leg west approach	
Signalized?: Weather:	No Clear and dry	
Vehicle Classifications:	Regular Vehicles Light Trucks Heavy Trucks Bicycles	This data is for All Vehicles Combined

Shift	Start	End	Duration
AM	7:00	9:00	2.00
MD			
PM	15:00	18:00	3.00
Total	7:00	18:00	5.00

 Notes:
 24-hour clock used for reporting (15-minute increments)

 North Approach - southbound vehicles approaching intersection from the north

 15x4 - 15 min volume (from maximum 15 minute period of movement/approach in peak hour period [*]) x 4

 Pedestrians - N indicates pedestrians crossing north approach (east/west)

Comments:



Survey Data
All Vehicles Combined

Time	ne Highway 1 High					High	way 1	1 Grouhel Rd																	
Period	NO	RTH		ach	SOUTH Approach						Approa		E/	AST A	Approa	ich	Total Vo 15-min	olume	äk	(Cross	walks			
Begins	Left		Right		Left		Right		Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	Ре	Ν	S	W	E		
7:00		146	5		1	216		217	11		2	13					381			0	0	0			
7:15		190	2		3	245		248	16		5	21					461			0	0	0			
7:30		227	4	231	2	270		272	8		6	14					517			0	0	0			
7:45		203	5		9	230		239	3		3	6					453	1812		0	0	0			
8:00		217	5		2	227		229	7		7	14					465	1896	*	0	0	0			
8:15		208	4	212	9	233		242	6		4	10					464	1899	*	0	0	0			
8:30		245 252	3	248 259	4	244 198		248	6 4		7 5	13 9					509	1891	+	0	0	0			
8:45 n/a		252	1	259	1	198		199	4		5	9					467	1905		0	0	0			
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15:00		322	4	326	7	319		326	6		9	15					667			0	0	0			
15:15		304	7	311	10	327		337	5		3	8					656			0	0	0			
15:30		373	10		11	349		360	4		3	7					750			0	0	0			
15:45		345	8	353	9	284		293	2		6	8					654	2727		0	0	0			
16:00		314	6	320	6	322		328	2		9	11					659	2719		0	0	0			
16:15		338	10		8	357		365	8		4	12					725	2788	*	0	0	0			
16:30		363	13	376	7	384		391	9		7	16					783	2821	*	0	0	0			
16:45		345	8		5	359		364	6		4	10					727	2894	*	0	0	0			
17:00		370	17	387	6	389		395	8		4	12					794	3029	+	0	0	0			
17:15		368	10	378	11	322		333	3		4	7					718	3022	\square	0	0	0			
17:30		342	13		3	265		268	2		6	8					631	2870	Ц	0	0	0			
17:45		323	16	339	12	271		283	2		6	8					630	2773	\square	0	0	0			
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n/a Total		1107	122	4220	95	3049		40.42	57		65	122					8394			0	0	0			
TOLA		4107	122	4229	95	3948		4043	5/		65	122					8394			U	0	0			



AM Peak Period All Vehicles Combined

Highway 1 @ Grouhel Road Thursday, August 9, 2018

Time		High				High					nel Rd														
Period	NO	RTH	Appro	ach	SO	UTH /	Approa	ach	W	EST /	Approa	ach	E/	AST A	pproa	ch	Total V 15-min	olume	ak	(Cross	walks		Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	Ре	Ν	S	W	Е	15 min	Hr
7:00		146	5	151	1	216		217	11		2	13					381			0	0	0		227	
7:15		190	2	192	3	245		248	16		5	21					461			0	0	0		261	
7:30		227	4		2	270		272	8		6	14					517			0	0	0		278	
7:45		203	5		9	230		239	3		3	6					453	1812		0	0	0		233	999
8:00		217	5	222	2	227		229	7		7	14					465	1896	*	0	0	0		234	1006
8:15		208	4	212	9	233		242	6		4	10					464	1899	*	0	0	0		239	984
8:30		245	3	248	4	244		248	6		7	13					509	1891	+	0	0	0		259	965
8:45		252	7	259	1	198		199	4		5	9					467	1905	*	0	0	0		265	997
n/a																									
n/a																									
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n/a																									
n/a																									
n/a																									
Total		1688	35	1723	31	1863		1894	61		39	100					3717		ſ	0	0	0		I I	1924
Avg Hr		844	18		16	932		947	31		20	50					1859		ł	0	0	0			1524
, wg in j		044	10	302	10	332		347	51		20	50					1009		L	0	0	0			
Peak h	our o	f the i	inters	ection	h																				

Peak hour of the intersection

Pk Hr	922	19	941	16	902	918	23	23	46			1905
15x4	1008	28	1036	36	976	992	28	28	56			2036
PHF	0.91	0.68	0.91	0.44	0.92	0.93	0.82	0.82	0.82			0.94

0	0	0		980
0	0	0		1100
n/a	n/a	n/a		0.89

0 0 0

0

n/a n/a n/a

0

0

1006

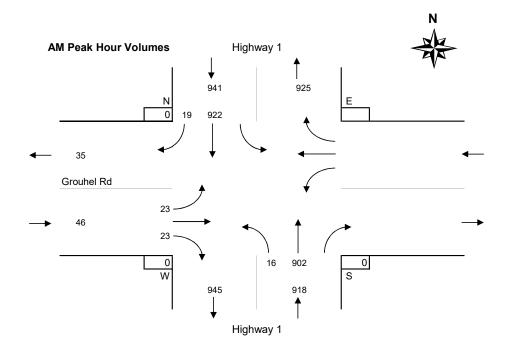
1144

0.88

Peak hour of conflicting volumes for the intersection

Pk Hr	837	16	853	16	972	988	34	21	55			1896
15x4	908	20	924	36	1080	1088	64	28	84			2068
PHF	0.92	0.80	0.92	0.44	0.90	0.91	0.53	0.75	0.65			0.92

** Calculated peak hour occurs during the first or last hour of shift and therefore may be invalid. **



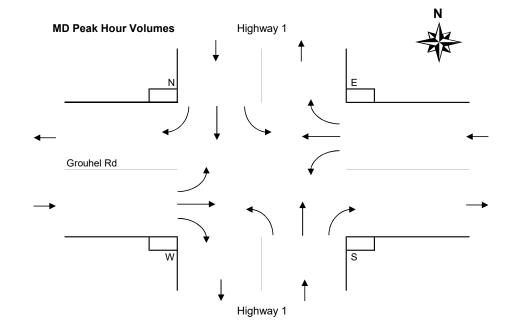


MD Peak Period All Vehicles Combined

PHF

Highway 1 @ Grouhel Road Thursday, August 9, 2018

Time		High	way 1			High	way 1			Grout	nel Rd						1								
Period	NO	RTH /	Appro	ach	SO	UTH	Appro	ach	W	EST A	Approa	ach	E/	AST A	pproa	ich	Total V	olume	ak		Cross	walks	6	Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Total V 15-min	Hour	Pe	Ν	S	W	Е	15 min	Hr
n/a																									
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							-	-																	
Total																									
Avg Hr																							_		
Peak h	our o	f the i	inters	ectior	<u>1</u>																				
Pk Hr																		*						ļl	
15x4																		+							
PHF] [
																			_					_	
Peak h	our o	f cont	flictin	g volu	umes	for th	e inte	rsect	ion										_						
Pk Hr																		*	[] [
15x4																		+] [
- OXI					-														- 1				_	4 H	





PM Peak Period All Vehicles Combined

Highway 1 @ Grouhel Road Thursday, August 9, 2018

Time		Highv	way 1			High	way 1			Grouł	nel Rd														
Period	NO	RTH /	Appro	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ich	Total V	olume	ak		Cross	walks		Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	Pe	Ν	S	W	Е	15 min	Hr
15:00		322	4	326	7	319		326	6		9	15					667			0	0	0		342	
15:15		304	7	311	10	327		337	5		3	8					656			0	0	0		332	
15:30		373	10	383	11	349		360	4		3	7					750			0	0	0		398	
15:45		345	8	353	9	284		293	2		6	8					654	2727		0	0	0		368	1440
16:00		314	6	320	6	322		328	2		9	11					659	2719		0	0	0		335	1433
16:15		338	10	348	8	357		365	8		4	12					725	2788	*	0	0	0		365	1466
16:30		363	13	376	7	384		391	9		7	16					783	2821	*	0	0	0		393	1461
16:45		345	8	353	5	359		364	6		4	10					727	2894	*	0	0	0		365	1458
17:00		370	17	387	6	389		395	8		4	12					794	3029	+	0	0	0		401	1524
17:15		368	10	378	11	322		333	3		4	7					718	3022		0	0	0		393	1552
17:30		342	13	355	3	265		268	2		6	8					631	2870		0	0	0		364	1523
17:45		323	16	339	12	271		283	2		6	8					630	2773		0	0	0		357	1515
n/a																									
n/a																									
n/a																									
n/a																									
n/a																									
n/a																									
n/a																									
n/a																									
Total		4107	122	4229	95	3948		4043	57		65	122					8394		L	0	0	0		i l	4389
Avg Hr		1369	41	1410	32	1316		1348	19		22	41					2798		l	0	0	0		ĺ	

Peak hour of the intersection

Pk Hr	1416	48	1464	26	1489	1515	31	19	50			3029
15x4	1480	68	1548	32	1556	1580	36	28	64			3176
PHF	0.96	0.71	0.95	0.81	0.96	0.96	0.86	0.68	0.78			0.95

0	0	0		1521
0	0	0		1616
n/a	n/a	n/a		0.94

0 0 0

0

n/a

0

n/a

0

n/a

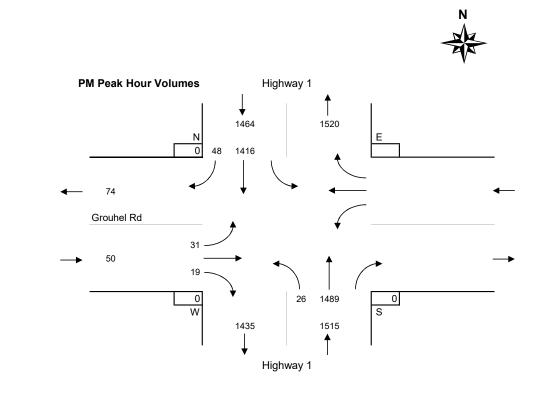
1549

1628

0.95

Peak hour of conflicting volumes for the intersection

1 Out I	 		9			•		0.11					
Pk Hr	1446	48	1494	29	1454		1483	26	19	45			3022
15x4	1480	68	1548	44	1556		1580	36	28	64			3176
PHF	0.98	0.71	0.97	0.66	0.93		0.94	0.72	0.68	0.70			0.95





Entire Survey Period

		High	way 1			High	way 1			Grouł	nel Rd							_				
	NO	RTH	Approa	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total	1[(Cross	walks	5
	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume		Ν	S	W	
Total		5795	157	5952	126	5811		5937	118		104	222					12111] [0	0	0	
Avg Hr		1159	31	1190	25	1162		1187	24		21	44					2422] [0	0	0	

AM Peak Period

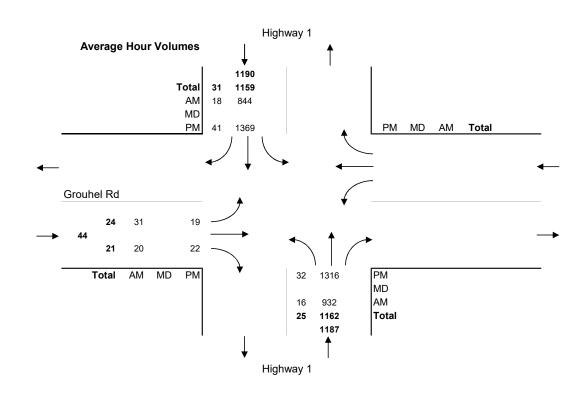
_																				
		High	way 1			High	way 1			Grouł	nel Rd							 _		
	NO	RTH .	Appro	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E	AST A	pproa	ch	Total	[0	Cro
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	[Ν	S
Period		1688	35	1723	31	1863		1894	61		39	100					3717		0	
Avg Hr		844	18	862	16	932		947	31		20	50					1859		0	

MD Peak Period

		High	way 1			High	way 1			Grouł	nel Rd							 				
	NORTH Approach			ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total		(Cross	walks	;
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume		N	S	W	Е
Total																						
Avg Hr																						

PM Peak Period

		High	way 1			High	way 1			Grouł	nel Rd										
	NO	RTH	Approa	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E	AST A	pproa	ich	Total	(Cross	walks	5
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	Ν	S	W	E
Total		4107	122	4229	95	3948		4043	57		65	122					8394	0	0	0	
Avg Hr		1369	41	1410	32	1316		1348	19		22	41					2798	0	0	0	





2 Hours

5 Hours

3 Hours

Hours

E

Highway 1 @ Grouhel Road Thursday, August 9, 2018

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osswalks S W Е



Major Route: Minor Route: Municipality: Filename: Location #:	Highway 1 Ludlow Road Ladysmith 2-Highway 1 @ Ludlow Rd-Aug 2	9, 2018.xlsx
Date: Day-of-week:	August 9, 2018 Thursday	
East/West Route: Intersection Type: Signalized?: Weather:	Ludlow Road 4-leg Yes Clear and dry	
Vehicle Classifications:	Regular Vehicles Light Trucks Heavy Trucks Bicycles	This data is for All Vehicles Combined

Shift	Start	End	Duration
AM	7:00	9:00	2.00
MD			
PM	15:00	18:00	3.00
Total	7:00	18:00	5.00

 Notes:
 24-hour clock used for reporting (15-minute increments)

 North Approach - southbound vehicles approaching intersection from the north

 15x4 - 15 min volume (from maximum 15 minute period of movement/approach in peak hour period [*]) x 4

 Pedestrians - N indicates pedestrians crossing north approach (east/west)

Comments:



Time		High	wav 1			Highv	vav 1			1st	Ave			Ludic	w Rd								
Period	NO		Appro	ach	SO	UTH A	Approa		W		Approa	ich	E/	AST A	Approa		Total Vo 15-min	olume	Ř	C	Cross		
Begins	Left		Right		Left		Right		Left	Thru	Right	Total	Left		Right			Hour	Ре	Ν	S	W	E
7:00	7	147	9	163	3	175	2	180	45	6	7	58	6	3			416			0	2	0	0
7:15 7:30	6 6	162 202	14 26	182 234	4	195 198	3 4	202 204	58 56	7 12	19 14	84 82	3	0		7 19	475 539		$\left \right $	0	1	0	0 0
7:45	8	170	20	204	2	196	4	176	66	8	6	 80	9	7			484	1914	$\left \right $	0	2	0	0
8:00	11	193	34	238	0	167	10	177	62	5	13	80	6				510	2008	*	0	0	0	0
8:15	11	170	25	206	6	172	9	187	63	9	10	82	5	2			489	2022	*	0	0	0	0
8:30	11	202	42	255	6	180	14	200	61	11	16	88	8	7			563	2046	+	0	0	0	0
8:45	11	200	45	256	6	151	18	175	39	14	9	62	13	9	12	34	527	2089	*	0	2	0	0
n/a																							
n/a n/a																			\vdash				
n/a																							
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n/a																							
Total	71	1446	221	1738	30	1404	67	1501	450	72	94	616	56	35	57	148	4003			0	7	0	0
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n/a n/a																							
n/a																							
n/a																			\vdash				
n/a																							
n/a																							
Total			_																				
15:00	10	257	46	313	4	247	14	265	57	19	13	89	19	21	15	55	722			0	2	0	0
15:15	11	237	40 56	302	4	284	14	302	56	19	13	- 89 79	20	21	13	55	722			0	2	0	0
15:30	5	324	53	382	2	287	22	311	38	16	12	64	20	15		64	821		\vdash	0	2	0	0
15:45	11	256	61	328	5	225	16	246	58	15	18	91	21	12	13	46	711	2992	\vdash	0	3	0	0
16:00	9	264	53	326	6	266	14	286	64	10	10	84	17	18		47	743	3013		0	2	0	0
16:15	8	258	58	324	7	288	14	309	62	8	9	79	10	17	8		747	3022	*	0	1	0	1
16:30	6	278	56	340	4	314	9	327	59	9	7	75	25	24	8		799	3000	*	0	0	0	0
16:45 17:00	16 8	277 288	75 53	368 349	2	332 292	10 15	344 311	60 74	12 7	7 9	79 90	14 21	21 17	4		830 791	3119 3167	+	0	0	0	0
17:00	8 7	288	53 57	349	4 5	292	10	289	65	8	9	90 79	21 17	9		33	791	3167	\vdash	0	0	0	0
17:30	6	279	64	349	6	214	10	235	51	6	5	62	12	9		33	679	3040	⊢	0	0	0	0
17:45	6	264	44	314	7	211	8	226	44	6	7	57	14	4		25	622	2832	\square	0	0	0	0
n/a																							
n/a																			Д				
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n/a																			\square				$ \longrightarrow $
n/a n/a																			\vdash				
n/a n/a																			\vdash				
n/a																			\square	-+			\neg
Total	103	3255	676	4034	55	3237	159	3451	688	127	113	928	215	189	126	530	8943			0	10	0	1



AM Peak Period All Vehicles Combined

Highway 1 @ Ludlow Road Thursday, August 9, 2018

Time		High	way 1			Highv				1st	Ave			Ludlo	w Rd		1								
Period	NO	RTH /	Appro	ach	SO	UTH	Appro	ach	W	EST A	Approa	ach	E/	AST A	pproa	ich	Total V 15-min	olume	ak	(Cross	walks		Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	Pe.	Ν	S	W	Е	15 min	Hr
7:00	7	147	9	163	3	175	2	180	45	6	7	58	6	3	6	15	416			0	2	0	0	238	
7:15	6	162	14	182	4	195	3	202	58	7	19	84	3	0	4	7	475			0	1	0	0	266	
7:30	6	202	26	234	2	198	4	204	56	12	14	82	6	3	10					0	2	0	0	299	
7:45	8	170	26	204	3	166	7	176	66	8	6	80	9	7	8	24		1914		0	0	0	0	280	1083
8:00	11	193	34	238	0	167	10	177	62	5	13	80	6	4	5	15		2008	*	0	0	0	0	298	1143
8:15	11	170	25	206	6	172	9		63	9	10	82	5	2		14		2022	*	0	0	0	0	273	1150
8:30	11	202	42	255		180	14	200	61	11	16	88	8	7	5	20		2046	+	0	0	0	0	323	1174
8:45	11	200	45	256	6	151	18	175	39	14	9	62	13	9	12	34	527	2089	*	0	2	0	0	311	1205
n/a																									
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Total	71	1446	221	1738	30	1404	67	4504	450	70	94	616	56	25	57	148	4002		Г		7	0	0		2220
Avg Hr	36	723	111	869	30 15		34		450 225	72 36	94 47	308	28	35 18					H	0	7	0	0	l	2239
~vg m	30	123	111	009	15	702	- 34	/51	225	30	47	308	20	10	29	74	2002	l	L	0	4	0	0		
Peak h	our o	f the i	inters	ectior	า																				

the intersection

Pk Hr	44	765	146	955	18	670	51	739	225	39	48	312	32	22	29	83	2089	*
15x4	44	808	180	1024	24	720	72	800	252	56	64	352	52	36	48	136	2252	+
PHF	1.00	0.95	0.81	0.93	0.75	0.93	0.71	0.92	0.89	0.70	0.75	0.89	0.62	0.61	0.60	0.61	0.93	

0	2	0	0	1205	
0	8	0	0	1348	
/a	0.25	n/a	n/a	0.89	

0

n/a

0 0

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2

1205

1348

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0

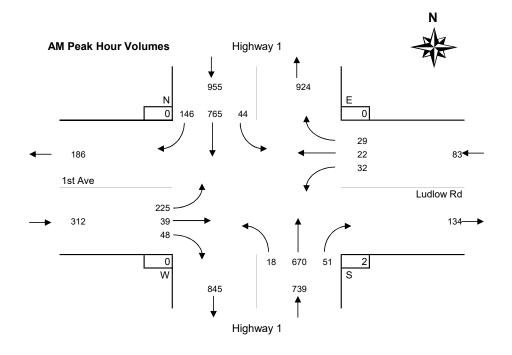
0 8 0

n/a 0.25

Peak hour of conflicting volumes for the intersection

Pk Hr	44	765	146	955	18	670	51	739	225	39	48	312	32	22	29	83	2089
15x4	44	808	180	1024	24	720	72	800	252	56	64	352	52	36	48	136	2252 -
PHF	1.00	0.95	0.81	0.93	0.75	0.93	0.71	0.92	0.89	0.70	0.75	0.89	0.62	0.61	0.60	0.61	0.93

** Calculated peak hour occurs during the first or last hour of shift and therefore may be invalid. **



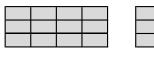


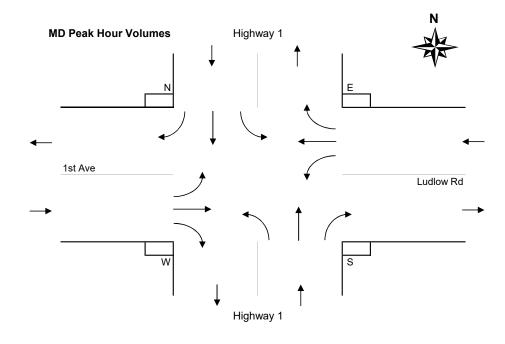
MD Peak Period All Vehicles Combined

Highway 1 @ Ludlow Road Thursday, August 9, 2018

Time		High	way 1			High	way 1			1st	Ave				w Rd										
Period	NO	RTH /	Appro	ach	SO	UTH	Approa	ach	W	EST A	Approa	ach	E	AST A	pproa	ich	Total V	olume	æ	(Cross	walks	5	Cor	flict
Begins			Right		Left		Right		Left	Thru	Right	Total	Left	Thru	Right	Total	Total V 15-min	Hour	Pe	Ν	S	W	Е	15 min	Hr
n/a																									
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																		1							
Total																									
Avg Hr																									
Peak h	our o	f the i	inters	ectior	ı																				
Pk Hr																		*						[
15x4																		+							
PHF																			Ì						
Peak h	our of conflicting volumes for the intersection						ion																		
Pk Hr																		*							

Pk Hr									*
15x4									+
PHF									





TRANS **F**(Data Ser

Time		High	way 1			Highv				1st	Ave			Ludlo	w Rd										
Period	NO	RTH /	Appro	ach	SO	UTH A	Approa	ach	W	EST A	Approa	ach	EA	AST A	pproa	ch	Total V	olume	ak		Cross	walks	;	Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	Pe	N	S	W	Е	15 min	Hr
15:00	10	257	46	313	4	247	14	265	57	19	13	89	19	21	15	55	722			0	2	0	0	400	
15:15	11	235	56		3	284	15	302	56	11	12	79	20	22	13	55	738			0	0	0	0	401	
15:30	5	324	53		2	287	22	311	38	16	10	64	25	15	24	64	821			0	2	0	0	456	
15:45	11	256	61		5	225	16	246	58	15	18	91	21	12	13	46		2992		0	3	0	0	405	1662
16:00	9	264	53		6	266	14	286	64	10	10	84	17	18	12	47	743	3013		0	2	0	0	417	1679
16:15	8	258	58	324	7	288	14	309	62	8	9	79	10	17	8	35		3022		0	1	0	1	410	1688
16:30	6	278	56		4	314	9	327	59	9	7	75	25	24	8	57	799	3000		0	0	0	0	429	1661
16:45	16	277	75		2	332	10	344	60	12	7	79	14	21	4	39	830	3119	+	0	0	0	0	443	1699
17:00	8	288	53		4	292	15	311	74	7	9	90	21	17	3	41	791	3167	*	0	0	0	0	439	1721
17:15	7	275	57	339	5	274	10	289	65	8	6	79	17	9	7	33	740	3160		0	0	0	0	418	1729
17:30	6	279	64	349	6	217	12	235	51	6	5	62	12	9	12	33		3040		0	0	0	0	421	1721
17:45	6	264	44	314	7	211	8	226	44	6	7	57	14	4	7	25	622	2832		0	0	0	0	370	1648
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Tatal	400	0055	070			0007	4.50			107	440		045	400	100		00.40		г		4.0			п	1000
Total	103	3255	676		55		159			127	113	928	215		126	530			-	0	10	0	1	L	4989
Avg Hr	34	1085	225	1345	18	1079	53	1150	229	42	38	309	72	63	42	177	2981		l	0	3	0	0		

Peak hour of the intersection

Pk	Hr	38	1101	242	1381	17	1226	48	1291	255	36	32	323	70	79	23	172	3167
15x	(4	64	1152	300	1472	28	1328	60	1376	296	48	36	360	100	96	32	228	3320
PH	-IF	0.59	0.96	0.81	0.94	0.61	0.92	0.80	0.94	0.86	0.75	0.89	0.90	0.70	0.82	0.72	0.75	0.95

0	1	0	1	1717
0	4	0	4	1904
n/a	0.25	n/a	0.25	0.90

n/a

0 0 0 0

0

n/a n/a n/a

0 0 0

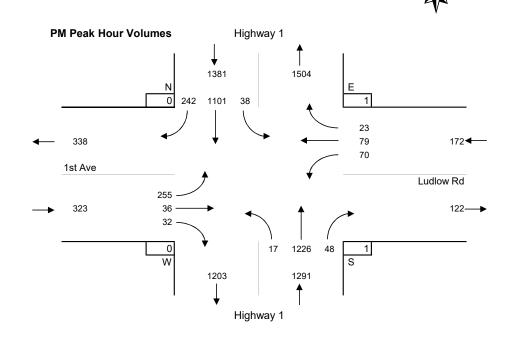
1725

1896

0.91

Peak hour of conflicting volumes for the intersection

Pk Hr	37	1118	241	1396	15	1212	44	1271	258	36	29	323	77	71	22	170	3160
15x4	64	1152	300	1472	20	1328	60	1376	296	48	36	360	100	96	32	228	3320 ·
PHF	0.58	0.97	0.80	0.95	0.75	0.91	0.73	0.92	0.87	0.75	0.81	0.90	0.77	0.74	0.69	0.75	0.95





Entire Survey Period

		High	way 1			High	way 1			1st	Ave			Ludlo	w Rd			 			
	NO	RTH	Appro	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total		Cross	walks	\$
	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	Ν	S	W	
Total	174	4701	897	5772	85	4641	226	4952	1138	199	207	1544	271	224	183	678	12946	0	17	0	
Avg Hr	35	940	179	1154	17	928	45	990	228	40	41	309	54	45	37	136	2589	0	3	0	Г

AM Peak Period

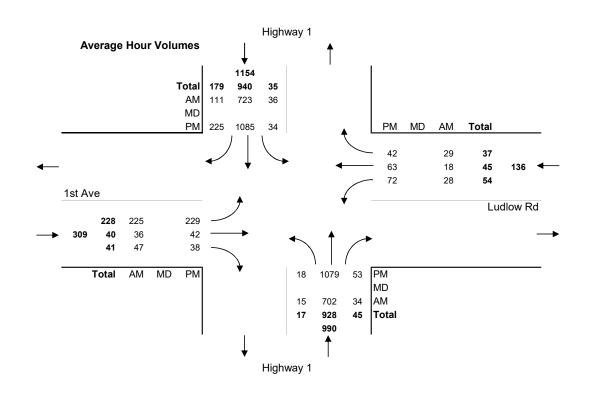
		High	way 1			High	way 1			1st	Ave			Ludlo	w Rd			 			
	NO	RTH	Appro	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total		Cross	walks	5
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	N	S	W	
Period	71	1446	221	1738	30	1404	67	1501	450	72	94	616	56	35	57	148	4003	0	7	0	Ē
Avg Hr	36	723	111	869	15	702	34	751	225	36	47	308	28	18	29	74	2002	0	4	0	

MD Peak Period

		High	way 1			High	way 1			1st	Ave			Ludlo	w Rd			 			
	NO	RTH	Appro	ach	SO	UTH /	Appro	ach	W	EST A	Approa	ach	E,	AST A	pproa	ich	Total		Cross	swalks	5
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	N	S	W	Е
Total																					
Avg Hr																					

PM Peak Period

		High	way 1			High	way 1			1st	Ave			Ludlo	w Rd							
	NO	RTH /	Approa	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total	10	(Cross	walks	;
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume		Ν	S	W	Е
Total	103	3255	676	4034	55	3237	159	3451	688	127	113	928	215	189	126	530	8943		0	10	0	1
Avg Hr	34	1085	225	1345	18	1079	53	1150	229	42	38	309	72	63	42	177	2981] [0	3	0	0





Highway 1 @ Ludlow Road Thursday, August 9, 2018

W Е 0 0 0

W Е 0 0

> 0 0

5 Hours

Hours

3 Hours

2 Hours



Major Route: Minor Route: Municipality: Filename: Location #:	Highway 1 Roberts Street Ladysmith 3-Highway 1 @ Roberts St-Aug 9 3	9, 2018.xlsx
Date:	August 9, 2018	
Day-of-week:	Thursday	
East/West Route:	Roberts Street	
Intersection Type:	4-leg	
Signalized?: Weather:	Yes Clear and dry	
Vehicle Classifications:	Regular Vehicles	This data is for All Vehicles Combined
	Light Trucks Heavy Trucks Bicycles	

Shift	Start	End	Duration
AM	7:00	9:00	2.00
MD			
PM	15:00	18:00	3.00
Total	7:00	18:00	5.00

 Notes:
 24-hour clock used for reporting (15-minute increments)

 North Approach - southbound vehicles approaching intersection from the north

 15x4 - 15 min volume (from maximum 15 minute period of movement/approach in peak hour period [*]) x 4

 Pedestrians - N indicates pedestrians crossing north approach (east/west)

Comments:



Time		High	wav 1			Highv	wav 1			Robe	rts St		Tran	nsfer E	Beach	Blvd	Ì						
Period	NO	RTH /		ach	SO	UTH	Approa	ach	W	EST A					pproa		Total V	olume	⁸	(Cross	walks	
Begins			Right		Left		Right		Left	Thru	Right		Left	Thru			Total V 15-min	Hour	Pe	N	S	W	Е
7:00	1	160	3		3	169	1	173	10	2	15	27	1	2		4	368			0	0	1	0
7:15	8	166	5		12	200	2	214	8	2	14		0	3	-	4	421			0	0	0	0
7:30	5	236	1		14	194	1	209	16	1	15		1	1		-	489			2	0	0	0
7:45	4	178	6	188	16	182	1	199	11	5	7	23	2	4			422	1700		0	0	0	0
8:00	3	184	5		19	175	4	198	12	4	16		1	1				1761	*	0	0	0	0
8:15	5	199	4	208	24	193	4	221	11	4	13	28	3	6			469	1809	* *	3	0	0	0
8:30 8:45	4 9	219 215	5 5	228 229	26 24	193 178	6 8	225 210	11 17	5 7	9 13	25 37	2	0			482 485	1802 1865	" +	0	2	0	1
0.45 n/a	9	215	5	229	24	1/0	0	210	17	1	13	37		4	3	9	400	1000	-	3	- 0	0	0
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Total	39	1557	34	1630	138	1484	27	1649	96	30	102	228	12	21	25	58	3565			8	2	1	1
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15:00	5	287	10	302	22	263	8	293	12	5	25	42	13	11	13	37	674			2	0	0	0
15:15	8	274	14	296	33	292	5	330	16	8	19	43	4	11		26	695		\vdash	0	0	0	0
15:30	6	350	7	363	37	280	8	325	13	5	20	38	4	6		23	749		\square	0	0	0	0
15:45	7	318	5		39	263	7	309	12	6	20	38	5	8		20	697	2815		0	0	0	0
16:00	8	299	11	318	36	283	12	331	21	4	17	42	4	9		22	713	2854		1	0	0	0
16:15	11	284	14	309	44	296	9	349	29	5	25	59	12	7			743	2902	*	7	7	2	5
16:30	5	304	6		24	312	11	347	15	4	28	47	9	8		24	733	2886	*	0	0	7	0
16:45	5	299	13	317	39	341	11	391	13	3	25	41	10	6			774	2963	+	3	0	0	0
17:00	8	308	8		35	287	8	330	16	3	15	34	8	5			717	2967	*	3	1	3	0
17:15	10	310	10		31	287	2	320	14	6	23	43	8	2			709	2933	\square	5	0	0	0
17:30	11	291	11	313	27	245	12	284	12	6	18	36	11	3			656 624	2856	\vdash	0	0	0	0
17:45 n/a	6	300	11	317	31	216	7	254	8	9	19	36	8	3	6	17	624	2706	\vdash	2	0	U	U
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Total	90	3624	120	3834	398	3365	100	3863	181	64	254	499	96	79	113	288	8484			23	8	12	5



AM Peak Period All Vehicles Combined

Highway 1 @ Roberts Street Thursday, August 9, 2018

Time		High	way 1			Highv	vay 1			Robe	rts St		Tran	sfer E	leach	Blvd								
Period	NO	RTH	Appro	ach	SO	UTH	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total V	olume 👌	a۲	Cros	sswall	S	Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	ĽN	1 S	W	E	15 min	Hr
7:00	1	160	3	164	3	169	1	173	10	2	15	27	1	2	1	4	368			0	0	0) 189	
7:15	8	166	5	179	12	200	2	214	8	2	14	24	0	3	1	4	421			0	0 () () 226	
7:30	5	236	1	242	14	194	1	209	16	1	15	32	1	1	4	6				2	0 () (
7:45	4	178	6	188	16	182	1	199	11	5	7	23	2	4	6	12	422	1700		-	0 () (/	908
8:00	3	184	5	192	19	175	4	198	12	4	16	32	1	1	5	7	429	1761 *		-	0 () (948
8:15	5	199	4	208	24	193	4	221	11	4	13		3	6	3	12	469	1809 *			0 () (969
8:30	4	219	5	228	26	193	6	225	11	5	9	25	2	0	2	4	482	1802 *		_	2 (266	963
8:45	9	215	5	229	24	178	8	210	17	7	13	37	2	4	3	9	485	1865 +		3	0 () (268	1010
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Total	39		34	1630	138		27	1649	96	30		228	12	21	25					8	2	1	4 I	1873
Avg Hr	20	779	17	815	69	742	14	825	48	15	51	114	6	11	13	29	1783			4	1	1		
Dealth		f Ala a S			_																			
Peak h																		1		_	_	_		

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Ρ	'k Hr	21	817	19	857	93	739	22	854	51	20	51	122	8	11	13	32	1865	*	6	
1	5x4	36	876	20	916	104	772	32	900	68	28	64	148	12	24	20	48	1940	+	12	
	PHF	0.58	0.93	0.95	0.94	0.89	0.96	0.69	0.95	0.75	0.71	0.80	0.82	0.67	0.46	0.65	0.67	0.96		0.50	(

5	2	0	1	1008
	8	0	4	1112
	0.25	n/a	0.25	0.91

4

2 0

n/a 0.25

6

0.50 0.25

8 0

12

1008

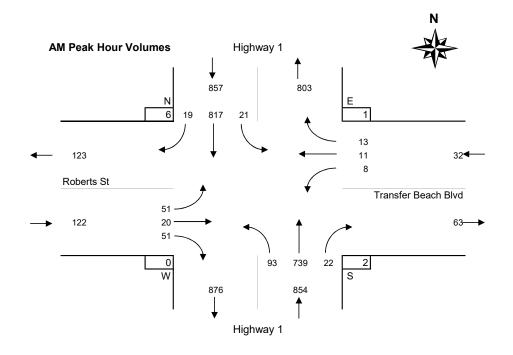
1112

0.91

Peak hour of conflicting volumes for the intersection

Pk Hr	21	817	19	857	93	739	22	854	51	20	51	122	8	11	13	32	1865
15x4	36	876	20	916	104	772	32	900	68	28	64	148	12	24	20	48	1940
PHF	0.58	0.93	0.95	0.94	0.89	0.96	0.69	0.95	0.75	0.71	0.80	0.82	0.67	0.46	0.65	0.67	0.96

** Calculated peak hour occurs during the first or last hour of shift and therefore may be invalid. **



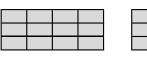


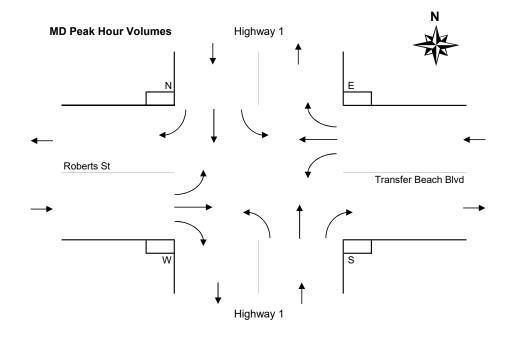
MD Peak Period All Vehicles Combined

Highway 1 @ Roberts Street Thursday, August 9, 2018

Time		High	way 1			High	way 1			Robe	erts St	Tran	isfer E	Beach	Blvd	1								
Period	NO		Appro	ach	SC	UTH	Appro	ach	W		Approa					Total V	olume	똪		Cross	walks	5	Cor	nflict
Begins	Left		Right		Left		Right		Left		Right		Thru	Right	Total	Total V 15-min	Hour	Pe	Ν	S	W	Е	15 min	Hr
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Peak h	our o	f the i	inters	ection	n																			
Pk Hr																	*							
15x4																	+							
PHF																								
																							-	
Peak h	our o	f con	flictin	g volu	umes	for th	e inte	rsecti	ion								-	_						
Pk Hr																	*							
4 - 4																								

Pk Hr									*
15x4									+
PHF									







PM Peak Period All Vehicles Combined

Highway 1 @ Roberts Street Thursday, August 9, 2018

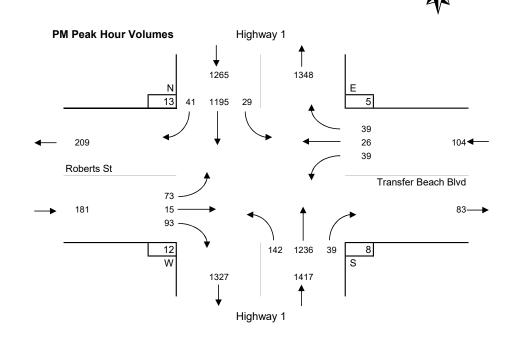
Time		High	way 1			High	way 1			Robe	rts St		Tran	isfer B	each	Blvd									
Period	NO	RTH /	Appro	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E	AST A	pproa	ch	Total V			(Cross	walks		Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	Pe	N	S	W	E	15 min	Hr
15:00	5	287	10	302	22	263	8	293	12	5	25	42	13	11	13	37	674			2	0	0	0	362	
15:15	8	274	14	296	33	292	5	330	16	8	19	43	4	11	11	26	695			0	0	0	0	359	
15:30	6	350	7	363	37	280	8	325	13	5	-	38	4	6	13	23	749			0	0	0	0	426	
15:45	7	318	5		39	263	7	309	12	6	20	38	5	8	7	20	697	2815		0	0	0	0	393	1540
16:00	8	299	11	318	36	283	12	331	21	4	17	42	4	9	9	22	713	2854		1	0	0	0	385	1563
16:15	11	284	14	309	44	296	9	349	29	5	25	59	12	7	7	26	743	2902	*	7	7	2	5	385	1589
16:30	5	304	6		24	312	11	347	15	4	28	47	9	8	7	24	733	2886		0	0	7	0	375	1538
16:45	5	299	13		39	341	11	391	13	3	25	41	10	6	9	25	774	2963	+	3	0	0	0	395	1540
17:00	8	308	8		35	287	8	330	16	3	15	34	8	5	16	29	717	2967	*	3	1	3	0	388	1543
17:15	10	310	10		31	287	2	320	14	6	23	43	8	2	6	16	709	2933		5	0	0	0	388	1546
17:30	11	291	11	313	27	245	12	284	12	6	18	36	11	3	9	23	656	2856		0	0	0	0	364	1535
17:45	6	300	11	317	31	216	7	254	8	9	19	36	8	3	6	17	624	2706		2	0	0	0	378	1518
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Total	90	3624	120	3834	398	3365	100	3863	181	64	254	499	96	79	113	288	8484		ſ	23	8	12	5	Г	4556
Avg Hr	30	1208	40	1278	133	1122	33	1288	60	21	85	166	30	26	38	96	2828		ł	23	3	4	2	L	+000
<u> </u>		. 200			.00									20		•••	1010		L	-	Ű		_		

Peak hour of the intersection

					-																	
Pk Hr	29	1195	41	1265	142	1236	39	1417	73	15	93	181	39	26	39	104	2967	*	13	8	12	5
15x4	44	1232	56	1296	176	1364	44	1564	116	20	112	236	48	32	64	116	3096	+	28	28	28	20
PHF	0.66	0.97	0.73	0.98	0.81	0.91	0.89	0.91	0.63	0.75	0.83	0.77	0.81	0.81	0.61	0.90	0.96		0.46	0.29	0.43	0.25

Peak hour of conflicting volumes for the intersection

				9			•		••••								
Pk Hr	32	1251	37	1320	156	1122	36	1314	75	20	82	177	25	30	36	91	2902
15x4	44	1400	56	1452	176	1184	48	1396	116	24	100	236	48	36	52	104	2996
PHF	0.73	0.89	0.66	0.91	0.89	0.95	0.75	0.94	0.65	0.83	0.82	0.75	0.52	0.83	0.69	0.88	0.97





0.29	0.25

8

28 28

1525 1676 0.91

2 5 20

8 0.25 0.25

1585
1836
0.86

Entire Survey Period

		High	<i>w</i> ay 1			High	way 1			Robe	erts St		Tran	sfer E	Beach	Blvd		 			
	NO	RTH	Appro	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total		Cros	swalks	s
	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	N	S	W	
Total	129	5181	154	5464	536	4849	127	5512	277	94	356	727	108	100	138	346	12049	3	1 10) 13	Γ
Avg Hr	26	1036	31	1093	107	970	25	1102	55	19	71	145	22	20	28	69	2410		6 2	2 3	Γ

AM Peak Period

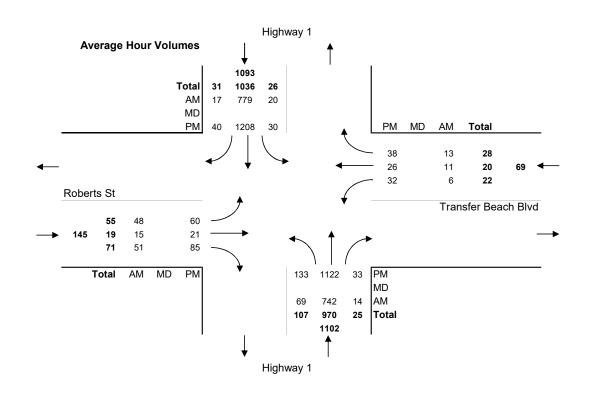
		High	way 1			High	way 1			Robe	erts St		Tran	nsfer E	Beach	Blvd					
	NO	RTH	Appro	ach	SOUTH Approach Left Thru Right Total			W	EST A	Approa	ach	E/	AST A	pproa	ich	Total		Cros	swalk	s	
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	N	S	W	
Period	39	1557	34	1630	138	1484	27	1649	96	30	102	228	12	21	25	58	3565		8 2	: 1	
Avg Hr	20	779	17	815	69	742	14	825	48	15	51	114	6	11	13	29	1783		4 1	1	Τ

MD Peak Period

		High	way 1			High	way 1			Robe	rts St		Tran	isfer B	leach	Blvd		 			
	NORTH Approach			ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total		Cross	walks	\$
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	N	S	W	Е
Total																					
Avg Hr																					

PM Peak Period

		High	<i>w</i> ay 1			High	way 1			Robe	rts St		Tran	sfer B	leach	Blvd						
	NORTH Approach			ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total	10	(Cross	walks	
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume		Ν	S	W	Е
Total	90	3624	120	3834	398	3365	100	3863	181	64	254	499	96	79	113	288	8484] [23	8	12	5
Avg Hr	30	1208	40	1278	133	1122	33	1288	60	21	85	166	32	26	38	96	2828] [8	3	4	2





W Е 13

6 1 3

Е

5 Hours

2 Hours

Hours

3 Hours



Major Route: Minor Route: Municipality: Filename: Location #:	Highway 1 N Davis Road Ladysmith 4-Highway 1 @ N Davis Rd-Aug 4	9, 2018.xlsx
Date:	August 9, 2018	
Day-of-week:	Thursday	
East/West Route:	N Davis Road	
Intersection Type:	4-leg	
Signalized?: Weather:	Yes Clear and dry	
Vehicle Classifications:	Regular Vehicles Light Trucks Heavy Trucks Bicycles	This data is for All Vehicles Combined

Shift	Start	End	Duration
AM	7:00	9:00	2.00
MD			
PM	15:00	18:00	3.00
Total	7:00	18:00	5.00

 Notes:
 24-hour clock used for reporting (15-minute increments)

 North Approach - southbound vehicles approaching intersection from the north

 15x4 - 15 min volume (from maximum 15 minute period of movement/approach in peak hour period [*]) x 4

 Pedestrians - N indicates pedestrians crossing north approach (east/west)

Comments:



Time		High	hway 1 Highway 1 I Approach SOUTH Approach							N Dav	/is Rd			N Day	vis Rd		1						
Period	NO	RTH /		ach	SO			ach			Approa	ach			Approa		Total V	olume	ak	(Cross	walks	
Begins				Total		Thru			Left	Thru	Right		Left	Thru			Total V 15-min	Hour	Pe	N	S	W	Е
7:00	8	147	19	174	7	134	0	141	26	2	24	52	0	4	18	22	389			0	0	0	0
7:15	18	144	18		9	151	0	160	25	3	25	53	2	4		41	434			0	0	0	0
7:30	19	222	19		8	171	3	182	14	0	20	34	5	4	47	56	532			1	0	0	0
7:45	14	170	16		13	132	2	147	19	2	23	44	4	8		49	440	1795		0	0	0	0
8:00	13	160	18		9	130	0	139	34	11	8	53	3			41	424	1830	*	1	0	0	0
8:15	16	163	31	210	11	162	2	175	29	4	23	56	3	8		53	494	1890	*	0	0	0	0
8:30	13 13	191	28 29		13 11	159 145	2	174	30 22	9 10	23 16	62 48	1	13 10	35	49	517	1875 1913	+	0	0	0	0
8:45 n/a	13	181	29	223	- 11	145	1	157	22	10	10	48	5	10	35	50	478	1913		3	0	0	0
n/a																							
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n/a																							
Total	114	1378	178	1670	81	1184	10	1275	199	41	162	402	23	59	279	361	3708			5	0	0	0
2/2															1								
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Total																							
15:00	49	216	49	314	32	210	4	246	32	22	32	86	3	17	41	61	707			1	0	0	1
15:15	36	232	58		32	245	4	286	39	19	34	92	3	23	36	62	766			0	0	0	0
15:30	44	268	52		28	258	8	294	48	11	30	89	4	14	43	61	808			0	0	0	0
15:45	49	228	64		20	214	6	240	42	32	32	106	2	18	37	57	744	3025		0	0	0	0
16:00	46	238	45	329	23	229	2	254	45	20	31	96	3	20	44	67	746	3064		0	0	0	0
16:15	47	223	49	319	33	297	4	334	45	17	23	85	3	15	35	53	791	3089	*	2	0	0	0
16:30	45	261	56	362	33	263	2	298	42	12	22	76	4	17	37	58	794	3075	*	1	0	0	1
16:45	46	219	64	329	32	307	10	349	39	10	29	78	0	23	44	67	823	3154	+	0	0	0	0
17:00	45	225	66		30	255	7	292	47	12	30	89	4	11	40	55	772	3180	*	3	0	0	0
17:15	62	243	63		25	235	10	270	42	13	35	90	0	9	36	45	773	3162		0	0	0	0
17:30	45	209	57	311	24	212	1	237	45	17	31	93	2	17	33	52	693	3061		0	0	0	0
17:45	41	210	64	315	32	195	1	228	36	16	37	89	1	8	21	30	662	2900		0	0	0	0
n/a																							-+
n/a n/a																				\vdash			-+
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n/a																							-+
n/a																							-+
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n/a																							
Total	555	2772	687	4014	351	2920	57	3328	502	201	366	1069	29	192	447	668	9079			7	0	0	2



AM Peak Period All Vehicles Combined

Highway 1 @ N Davis Road Thursday, August 9, 2018

Time		High	way 1			High	way 1			N Dav	∕is Rd			N Dav	/is Rd		1								
Period	NO	RTH	Appro	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total V 15-min	olume	ak	(Cross	walks		Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	Pe	N	S	W	Е	15 min	Hr
7:00	8	147	19	174	7	134	0	141	26	2	24	52	0	4	18	22	389			0	0	0	0	221	
7:15	18	144	18	180	9	151	0	160	25	3	25	53	2	4	35	41	434			0	0	0	0	235	
7:30	19	222	19	260	8	171	3	182	14	0	20	34	5	4	47	56	532			1	0	0	0	314	
7:45	14	170	16	200	13	132	2		19		23	44	4	8	37	49	-	1795		0	0	0	0	263	1033
8:00	13	160	18	191	9	130	0		34	11	8	53	3	8	30	41		1830	*	1	0	0	0	259	1071
8:15	16	163	31	210	11	162	2		29	4	23	56	3	8	42	53		1890	*	0	0	0	0	284	1120
8:30	13	191	28	232	13	159	2		30		23	62	1	13	35	49		1875	_	0	0	0	0	310	1116
8:45	13	181	29	223	11	145	1	157	22	10	16	48	5	10	35	50	478	1913	*	3	0	0	0	288	1141
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T . 4 . 1			(=0											= 0					r.						a (=)
Total	114	1378	178	1670	81	1184	10		199		162	402	23	59	279	361	3708			5	0	0	0	i L	2174
Avg Hr	57	689	89	835	41	592	5	638	100	21	81	201	12	30	140	181	1854		L	3	0	0	0	ł	
Peak h	ouro	fthe	intore	octio	1																				
Peak II	55	695		856		596	5	645	115	34	70	219	12	39	142	193	1913	*	ſ	4	0	0	0	I I	1141
I K I II	55	095	100	000	44	390	5	045	115	- 34	70	219	12	39	142	193	1913		- H	4	0	0	0	i k	1141

Pk Hr	55	695	106	856	44	596	5	645	115	34	70	219	12	39	142	193	1913	*	4	
15x4	64	764	124	928	52	648	8	700	136	44	92	248	20	52	168	212	2068	+	12	
PHF	0.86	0.91	0.85	0.92	0.85	0.92	0.63	0.92	0.85	0.77	0.76	0.88	0.60	0.75	0.85	0.91	0.93		0.33	n/

4 0 0 0 12 0 0 0 0 0.33 n/a n/a n/a

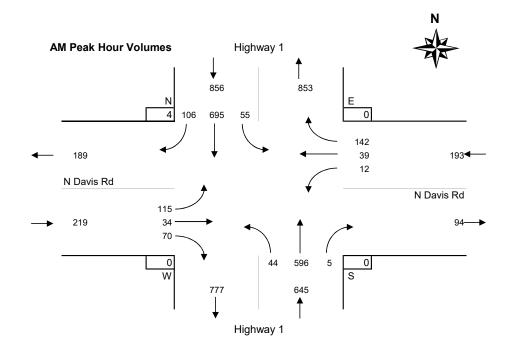
4	0	0	0	1141
12	0	0	0	1296
0.33	n/a	n/a	n/a	0.88

1296 0.88

Peak hour of conflicting volumes for the intersection

Pk Hr	55	695	106	856	44	596	5	645	115	34	70	219	12	39	142	193	1913
15x4	64	764	124	928	52	648	8	700	136	44	92	248	20	52	168	212	2068
PHF	0.86	0.91	0.85	0.92	0.85	0.92	0.63	0.92	0.85	0.77	0.76	0.88	0.60	0.75	0.85	0.91	0.93

** Calculated peak hour occurs during the first or last hour of shift and therefore may be invalid. **



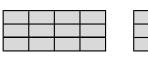


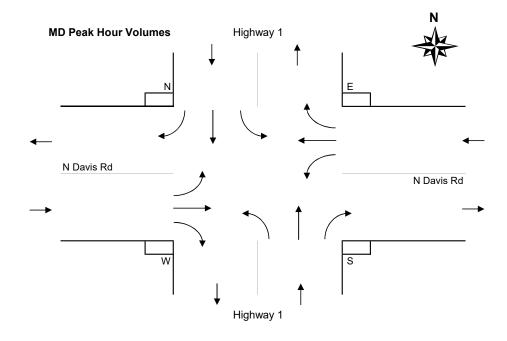
MD Peak Period All Vehicles Combined

Highway 1 @ N Davis Road Thursday, August 9, 2018

Time		High	way 1			High	way 1				vis Rd				∕is Rd										
Period	NO	RTH	Appro	ach	SO		Approa		W	EST A	Approa	ach	E/	AST A	pproa	ich	Total V	olume	ak	(Cross	walks	;	Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Total V 15-min	Hour	Pe	Ν	S	W	Е	15 min	Hr
n/a																									
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Total																									
Avg Hr																									
Peak h	our o	f the i	inters	ectior	h																				
Pk Hr					-													*	ſ					1 [
15x4																		+						i t	
PHF																			Ì						
Deale			6 11 - 41																						
Peak h	our o	t con	flictin	g volu	imes	for th	e inte	rsect	on									l.	r					1 r	
Pk Hr																		*	- 1						

Pk Hr									*
15x4									+
PHF									







PM Peak Period All Vehicles Combined

Highway 1 @ N Davis Road Thursday, August 9, 2018

Time		High	way 1			Highv	vay 1			N Dav	∕is Rd			N Dav	/is Rd		l l								
Period	NO	RTH /	Approa	ach	SO	UTH A	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total V		æ	0	Cross	walks		Cor	nflict
Begins	Left	Thru	Right	Total	15-min	Hour	Pe	Ν	S	W	Е	15 min	Hr												
15:00	49	216	49	314	32	210	4	246	32	22	32	86	3	17	41	61	707			1	0	0	1	387	
15:15	36	232	58	326	39	245	2	286	39	19	34	92	3	23	36	62	766			0	0	0	0	427	
15:30	44	268	52	364	28	258	8	294	48	11	30	89	4	14	43	61	808			0	0	0	0	453	
15:45	49	228	64	341	20	214	6	240	42	32	32	106	2	18	37	57	744	3025		0	0	0	0	409	1676
16:00	46	238	45	329	23	229	2	254	45	20	31	96	3	20	44	67	746	3064		0	0	0	0	415	1704
16:15	47	223	49	319	33	297	4	334	45	17	23	85	3	15	35	53		3089	*	2	0	0	0	443	1720
16:30	45	261	56	362	33	263	2	298	42	12	22	76	4	17	37	58		3075	*	1	0	0	1	446	1713
16:45	46	219	64	329	32	307	10	349	39	10	29	78	0	23	44	67	823	3154	+	0	0	0	0	469	1773
17:00	45	225	66	336	30	255	7	292	47	12	30	89	4	11	40	55		3180	*	3	0	0	0	419	1777
17:15	62	243	63	368	25	235	10	270	42	13	35	90	0	9	36	45		3162		0	0	0	0	418	1752
17:30	45	209	57	311	24	212	1	237	45	17	31	93	2	17	33	52	693	3061		0	0	0	0	385	1691
17:45	41	210	64	315	32	195	1	228	36	16	37	89	1	8	21	30	662	2900		0	0	0	0	371	1593
n/a																									
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n/a																									
n/a																									
n/a																									
n/a																									
Total	555	2772	687	4014	351	2920	57	3328	502	201	366	1069	29	192	447	668	9079			7	0	0	2		4951
Avg Hr	185	924	229	1338	117	973	19	1109	167	67	122	356	10	64	149	223	3026			2	0	0	1		

Peak hour of the intersection

Pk Hr	183	928	235	1346	128	1122	23	1273	173	51	104	328	11	66	156	233	3180	*	
15x4	188	1044	264	1448	132	1228	40	1396	188	68	120	356	16	92	176	268	3292	+	•
PHF	0.97	0.89	0.89	0.93	0.97	0.91	0.58	0.91	0.92	0.75	0.87	0.92	0.69	0.72	0.89	0.87	0.97		0.5

6	0	0	1	1723
12	0	0	4	1912
0.50	n/a	n/a	0.25	0.90

4

n/a 0.25

6 0 0

0 0

n/a

12

0.50

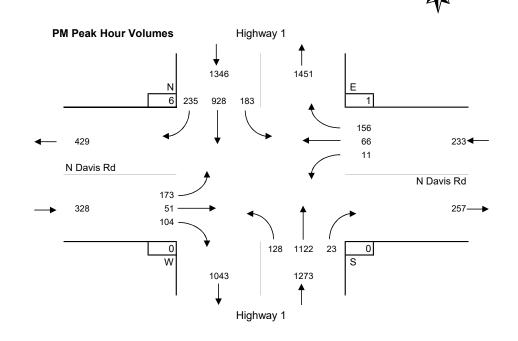
1723

1912

0.90

Peak hour of conflicting volumes for the intersection

Pk Hr	183	928	235	1346	128	1122	23	1273	173	51	104	328	11	66	156	233	3180
15x4	188	1044			-	1228	-	-	188	-				92	176		3292
PHF	0.97	0.89	0.89	0.93	0.97	0.91	0.58	0.91	0.92	0.75	0.87	0.92	0.69	0.72	0.89	0.87	0.97





Entire Survey Period

		High	way 1			High	way 1			N Dav	∕is Rd			N Dav	∕is Rd						
	NO	RTH /	Appro	ach	SO	UTH /	Approa	ach	WEST Approach			E/	AST A	pproa	ch	Total	(Cross	walks	;	
	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	Ν	S	W	
Total	669	4150	865	5684	432	4104	67	4603	701	242	528	1471	52	251	726	1029	12787	12	0	0	
Avg Hr	134	830	173	1137	86	821	13	921	140	48	106	294	10	50	145	206	2557	2	0	0	

AM Peak Period

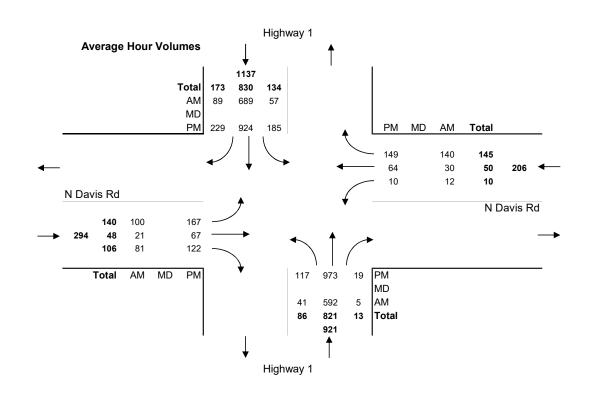
		High	way 1			High	way 1			N Dav	∕is Rd			N Dav	vis Rd						
	NO	RTH	Appro	ach	SO	UTH /	Appro	ach	W	EST A	Approa	ach	E/	AST A	Approa	nch	Total		(Cross	walł
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume		Ν	S	W
Period	114	1378	178	1670	81	1184	10	1275	199	41	162	402	23	59	279	361	3708		5	0	
Avg Hr	57	689	89	835	41	592	5	638	100	21	81	201	12	30	140	181	1854		3	0	(

MD Peak Period

		High	way 1			High	way 1			N Davis Rd WEST Approach				N Dav	∕is Rd			 			
	NO	RTH	Appro	ach	SO	UTH /	Approa	ach	W				E,	AST A	pproa	ich	Total	(Cross	walks	\$
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	Ν	S	W	Е
Total																					
Avg Hr																					

PM Peak Period

N Davis Rd Highway 1 Highway 1 N Davis Rd SOUTH Approach WEST Approach EAST Approach Total Crosswalks NORTH Approach Totals Thru Right Total Volume S W Left Thru Right Total Left Left Thru Right Total Left Thru Right Total Ν Total 555 2772 687 4014 351 2920 57 3328 502 201 366 1069 29 192 447 668 9079 0 0 19 Avg Hr 185 924 229 1338 117 973 1109 167 67 122 356 10 64 149 223 3026 2 0 0



Highway 1 @ N Davis Road Thursday, August 9, 2018

W Е 0 2 0 0

> E 0 0

٥ 0

5 Hours

Hours

3 Hours

2 Hours

Е 2 1



Major Route: Minor Route: Municipality: Filename: Location #:	Highway 1 Davis Road Ladysmith 5-Highway 1 @ Davis Rd-Aug 9, 5	2018.xlsx
Date:	August 9, 2018	
Day-of-week:	Thursday	
East/West Route:	Davis Road	
Intersection Type:	4-leg	
Signalized?: Weather:	No Clear and dry	
Vehicle Classifications:	Regular Vehicles Light Trucks Heavy Trucks Bicycles	This data is for All Vehicles Combined

Shift	Start	End	Duration
AM	7:00	9:00	2.00
MD			
PM	15:00	18:00	3.00
Total	7:00	18:00	5.00

 Notes:
 24-hour clock used for reporting (15-minute increments)

 North Approach - southbound vehicles approaching intersection from the north

 15x4 - 15 min volume (from maximum 15 minute period of movement/approach in peak hour period [*]) x 4

 Pedestrians - N indicates pedestrians crossing north approach (east/west)

Comments:



Time		High	wav 1			High	wav 1			Davi	s Rd			Davi	is Rd								
Period	NO	RTH		ach	SO		Approa	ach	W	EST A	Approa		E		Approa		Total V	olume	8 K	(Cross	walks	
Begins	Left		Right		Left		Right		Left	Thru	Right		Left	Thru		Total	Total V 15-min	Hour	Pe	Ν	S	W	Е
7:00	1	176	4		2	127	0	129	4	0	15	19	0	0			329			0	0	0	0
7:15	1	167	3		4	173	0	177	6	0	12	18	2	0			368		*	0	0	0	0
7:30 7:45	2	240 192	3	245 195	0	176 139	0	176 143	1 5	0 0	16	17 14	0	0		-	438 353	1400	+	0	0	0	0
8:00	1	192	2		6	159	0	143	5	0	9 7	8	0				345	1488 1504	*	0	0	0	0
8:15	1	183	4	188	5	148	0	153	3	0	9	12	1	0			354	1490		0	0	0	0
8:30	1	204	5		1	177	1	179	2	0	11	13	0				402	1454		0	0	0	0
8:45	1	202	5	208	2	165	0	167	0	0	12	12	0	0	0	0	387	1488		0	0	1	0
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Total	9	1541	29	1579	23	1255	2	1280	22	0	91	113	4	0	0	4	2976			0	0	1	0
n/a															1					- 1			
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15:45	1		10		15	257	1	273	. 1	1	12	14	0	0		-	582	2269		0	0	0	0
16:00	1	249	8		21	268	0	289	1	0	10	11	0	0	0	0	558	2278		0	0	0	0
16:15	4	238	11	253	17	296	2	315	1	0	7	8	0	0		-	576	2274		0	0	0	0
16:30	2	258	17	277	26	321	0	347	3	0	11	14	0	1		2	640	2356	+	0	0	0	0
16:45	1	233	12	246	31	336	0	367	4	0	12	16	0	0			629	2403	*	0	0	0	0
17:00	3	245	13		14	282	0	296	2	0	11	13	0	0		-	570	2415	Ĥ	0	0	0	0
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Total	21	2996	141	3158	213	3302	ð	3523	20	2	106	128	0	2	2	4	6813			0	0	0	0



AM Peak Period All Vehicles Combined

Highway 1 @ Davis Road Thursday, August 9, 2018

Time		High				Highv	vay 1			Davi				Davi	s Rd										
Period	NO	RTH /	Appro	ach	SO	UTH A	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total V	olume	æ	(Cross	walks		Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	ы Ре	N	S	W	Е	15 min	Hr
7:00	1	176	4	181	2	127	0	129	4	0	15	19	0	0	0	0	329			0	0	0	0	197	
7:15	1	167	3	171	4	173	0	177	6	0	12	18	2	0	0	2	368	•	*	0	0	0	0	188	
7:30	2	240	3	245	0	176	0	176	1	0	16	17	0	0	0	0	438		+	0	0	0	0	259	
7:45	1	192	2	195	3	139	1	143	5	0	9	14	1	0	0	1	353	1488	*	0	0	0	0	207	851
8:00	1	177	3	181	6	150	0	156	1	0	7	8	0	0	0	0	345	1504	*	0	0	0	0	193	847
8:15	1	183	4	188	5	148	0	153	3	0	9	12	1	0	0	1	354	1490		0	0	0	0	202	861
8:30	1	204	5	210	1	177	1	179	2	0	11	13	0	0	0	0	402	1454		0	0	0	0	221	823
8:45	1	202	5	208	2	165	0	167	0	0	12	12	0	0	0	0	387	1488		0	0	1	0	221	837
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Avg Hr	5 771 15 790 12 628						1	640	11	0	46	57	2	0	0	2	1488			0	0	1	0		
Peak h	our o	f the	inters	ectior	1																				

Pk Hr	5	776	11	792	13	638	1	652	13	0	44	57	3	0	0	3	1504
15x4	8	960	12	980	24	704	4	708	24	0	64	72	8	0	0	8	1752
PHF	0.63	0.81	0.92	0.81	0.54	0.91	0.25	0.92	0.54	n/a	0.69	0.79	0.38	n/a	n/a	0.38	0.86

0	0	0	0	847	
0	0	0	0	1068	
n/a	n/a	n/a	n/a	0.79	

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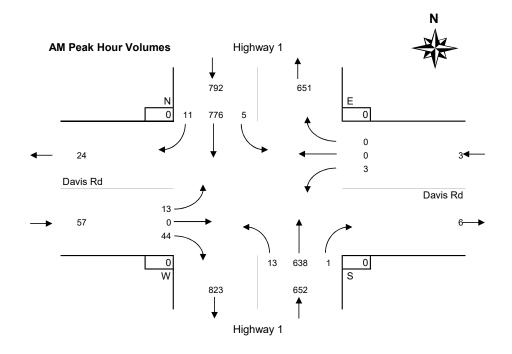
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0.81

Peak hour of conflicting volumes for the intersection

Pk Hr	5	792	12	809	14	613	1	628	10	0	41	51	2	0	0	2	1490
15x4	8	960	16	980	24	704	4	704	20	0	64	68	4	0	0	4	1752
PHF	0.63	0.83	0.75	0.83	0.58	0.87	0.25	0.89	0.50	n/a	0.64	0.75	0.50	n/a	n/a	0.50	0.85





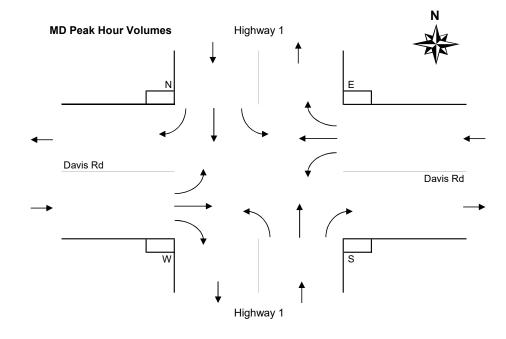
MD Peak Period All Vehicles Combined

Highway 1 @ Davis Road Thursday, August 9, 2018

Time		High				High	way 1				is Rd				s Rd										
Period	NO	RTH /	Appro	ach	SO	UTH /	Appro	ach	W		Approa		E/	AST A	pproa	ich	Total V 15-min	olume	ak	(Cross	walks	\$	Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	Pe.	Ν	S	W	E	15 min	Hr
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Peak h	k hour of conflicting vol		g volu	umes	for th	e inte	rsecti	ion																	
Pk Hr	Hr H H																	*	ſ] [
																	1		- B					1 H	

IKIII									
15x4									+
PHF									







PM Peak Period All Vehicles Combined

Time		High	<i>w</i> ay 1			Highv				Davi				Davi	s Rd		l l								
Period	NO	RTH /	Appro	ach	SO	UTH A	Approa	ach	W		Approa		E/		pproa	ch	Total V	olume	ak	(Cross	walks	;	Cor	ıflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	Pe	N	S	W	Е	15 min	Hr
15:00	1	248	12	261	10	269	1	280	2	0	6	8	0	0	0	0	549			0	0	0	0	277	
15:15	1	254	11	266	18	280	2	300	2	1	10	13	0	1	0	1	580			0	0	0	0	294	
15:30	1	257	11		14	266	0	280	1	0	8	9	0	0	0	0	558			0	0	0	0	290	
15:45	1	284	10		15	257	1	273	1	1	12	14	0	0	0	0	582	2269		0	0	0	0	322	1183
16:00	1	249	8		21	268	0	289	1	0	10	11	0	0	0	0	558	2278		0	0	0	0	288	1194
16:15		4 238 11 253 17 296 2 2 258 17 277 26 321 0					315	1	0	7	8	0	0	0	0	576	2274		0	0	0	0	309	1209	
16:30		2 258 17 277 26 321 (-	347	3	0		14	0	1	1	2	640	2356		0	0	0	0	334	1253
16:45	1	233	12	246	31	336	0	367	4	0		16	0	0	0	0	629	2403		0	0	0	0	349	1280
17:00	3	245	13		14	282	0	296	2	0	11	13	0	0	0	0	570	2415	*	0	0	0	0	296	1288
17:15	2	256	12		17	258	0	275	0	0		6	0	0	1	1	552	2391		0	0	0	0	291	1270
17:30	2	255			17	223	2	242	2	0		9	0	0	0	0		2272		0	0	0	0	292	1228
17:45	2	219	11	232	13	246	0	259	1	0	6	7	0	0	0	0	498	2141		0	0	0	0	254	1133
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Total	21	2996	141	3158	213	3302	8	3523	20	2	106	128	0	2	2	4	6813		ſ	0	0	0	0	Г	3458
Avg Hr	7	999	47	1053	71	1101	3	1174	7	1	35	43	0		1	1	2271			0	0	0	0	L	0.00
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Peak hour of the intersection

Pk Hr	10	974	53	1037	88	1235	2	1325	10	0	41	51	0	1	1	2	2415
15x4	16	1032	68	1108	124	1344	8	1468	16	0	48	64	0	4	4	8	2560
PHF	0.63	0.94	0.78	0.94	0.71	0.92	0.25	0.90	0.63	n/a	0.85	0.80	n/a	0.25	0.25	0.25	0.94

0 0 0 0 1288 0 0 0 0 1416 n/a n/a n/a n/a 0.91

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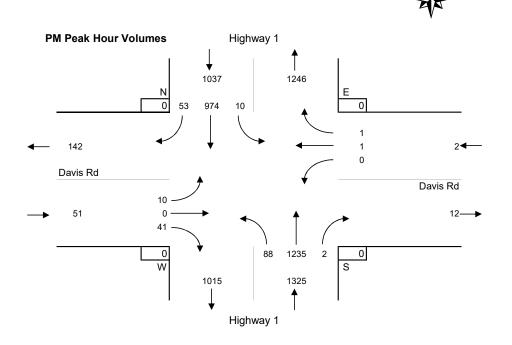
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Peak hour of conflicting volumes for the intersection

Pk Hr	10	974	53	1037	88	1235	2	1325	10	0	41	51	0	1	1	2	2415
15x4	16	1032	68	1108	124	1344	8	1468	16	0	48	64	0	4	4	8	2560
PHF	0.63	0.94	0.78	0.94	0.71	0.92	0.25	0.90	0.63	n/a	0.85	0.80	n/a	0.25	0.25	0.25	0.94





Entire Survey Period

		High	way 1			High	way 1			Davi	s Rd			Davi	is Rd			 			
	NO	RTH /	Appro	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E	AST A	Approa	ich	Total		Cross	walks	3
	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	Ν	S	W	
Total	30	4537	170	4737	236	4557	10	4803	42	2	197	241	4	2	2	8	9789	0	0	1	
Avg Hr	6	907	34	947	47	911	2	961	8	0	39	48	1	0	0	2	1958	0	0	0	

AM Peak Period

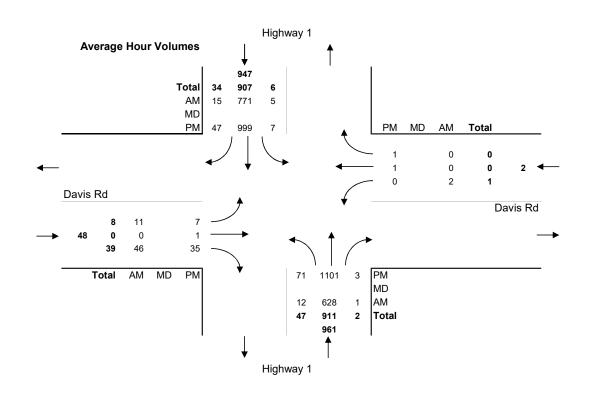
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		High	way 1			High	way 1			Davi	s Rd			Davi	s Rd						
	NO	RTH	Appro	ach	SOUTH Approach Left Thru Right Total			W	EST A	Approa	ach	E	AST A	pproa	ich	Total		(Cr	0	
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume		N		S
Period	9	1541	29	1579	23	1255	2	1280	22	0	91	113	4	0	0	4	2976		0		
Avg Hr	5	771	15	790	12	628	1	640	11	0	46	57	2	0	0	2	1488		0		

MD Peak Period

		High	way 1			Highway 1 SOUTH Approach				Davi	s Rd			Davi	s Rd			 				
	NO	RTH	Appro	ach	SO				W	EST A	Approa	ach	E/	AST A	pproa	ch	Total		C	Cross	walks	,
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	L L	1	S	W	Е
Total																						
Avg Hr																						

PM Peak Period

		High	way 1							Davi	s Rd			Davi	s Rd						
	NO	RTH /	Approa	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total		Cross	swalks	;
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	N	S	W	Е
Total	21	2996	141	3158	213	3302	8	3523	20	2	106	128	0	2	2	4	6813	(0 0	0	0
Avg Hr	7	999	47	1053	71	1101	3	1174	7	1	35	43	0	1	1	1	2271	(0 0	0	0





Highway 1 @ Davis Road Thursday, August 9, 2018

5 Hours

2 Hours

Hours

(Cross	walks	\$
Ν	S	W	Е
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3 Hours



Major Route: Minor Route: Municipality: Filename: Location #:	Highway 1 Edgelow Road Ladysmith 6-Highway 1 @ Edgelow Rd-Aug 6	g 9, 2018.xlsx
Date:	August 9, 2018	
Day-of-week: East/West Route:	Thursday Edgelow Road	
Intersection Type: Signalized?:	4-leg Yes	
Weather:	Clear and dry	
Vehicle Classifications:	Regular Vehicles Light Trucks Heavy Trucks Bicycles	This data is for All Vehicles Combined

Shift	Start	End	Duration
AM	7:00	9:00	2.00
MD			
PM	15:00	18:00	3.00
Total	7:00	18:00	5.00

 Notes:
 24-hour clock used for reporting (15-minute increments)

 North Approach - southbound vehicles approaching intersection from the north

 15x4 - 15 min volume (from maximum 15 minute period of movement/approach in peak hour period [*]) x 4

 Pedestrians - N indicates pedestrians crossing north approach (east/west)

Comments:



Period NORTH Approach SEAT Parate Total Total Total Total No Seat No Seat No <th< th=""><th>Time</th><th></th><th>High</th><th>way 1</th><th></th><th></th><th>High</th><th>way 1</th><th></th><th></th><th>Thick</th><th>ke Rd</th><th></th><th></th><th>Edgel</th><th>ow Ro</th><th>1</th><th>İ</th><th></th><th></th><th></th><th></th><th></th><th></th></th<>	Time		High	way 1			High	way 1			Thick	ke Rd			Edgel	ow Ro	1	İ						
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8:15 2 198 2 198 2 196 1 19 1 0 2 3 368 188 0																				*		_		
8:30 0 0:99 2:11 2:150 0:161 3:1 0:4 0:0 1 1 377 1430 0:0 0:0 0																				*				
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n/a a				5											Ť	<u> </u>			_ / 0 _	\vdash		Ť		Ť
n/a																				\square				
n/a a a a b a b a b																				Η				
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n/a	n/a																							
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Total 12 2984 28 3024 42 3369 8 3419 115 4 7 126 2 1 13 16 6585 0																								
	Total	12	2984	28	3024	42	3369	8	3419	115	4	7	126	2	1	13	16	6585			0	0	0	0

AM Peak Period All Vehicles Combined

Highway 1 @ Edgelow Road Thursday, August 9, 2018

Time		High				High					ke Rd			Edgel											
Period	NO	RTH /	Appro	ach	SO	UTH /	Approa	ach	W	EST /	Approa	hch	E/	AST A	pproa	ich	Total V	olume	ak	(Cross	walks		Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	Pe	Ν	S	W	E	15 min	Hr
7:00	0	181	6	187	1	140	0	141	1	0	0	1	0	0	0	0	329			0	0	0	0	189	
7:15	0	179	2	181	2	162	0	164	1	0	0	1	0	-	0	0	346			0	0	0	0	184	
7:30	0	252	3	255	1	170	2	173	4	0	0	4	2	0	1	3	435		+	0	0	0	0	261	
7:45	0	199	0	199	0	138	0	138	3	0	0	3	1	0	0	1	341	1451	*	0	0	0	0	202	836
8:00	2	183	1	186	1	150	0	151	6	0	0	6	0	0	1	1	344	1466	*	0	0	0	0	192	839
8:15	2	185	2	189	2	164	1	167	8	0	1	9	1	0	2	3	368	1488	*	0	0	0	0	199	854
8:30	0	209	2	211	2	159	0	161	3	1	0	4	0		1	1	377	1430		0	0	0	0	217	810
8:45	0	204	3	207	4	163	0	167	6	0	0	6	0	0	1	1	381	1470		0	0	0	0	218	826
n/a																									
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n/a																									
n/a																									
Total	4	1592	19		13		3	1262	32	1	1	34	4							0	0	0	0	l	1662
Avg Hr	2	796	10	808	7	623	2	631	16	1	1	17	2	0	3	5	1461			0	0	0	0		
		.																							
Peak h																									

Pk Hr	4	819	6	829	4	622	3	629	21	0	1	22	4	0	4	8	1488
15x4	8	1008	12	1020	8	680	8	692	32	0	4	36	8	0	8	12	1740
PHF	0.50	0.81	0.50	0.81	0.50	0.91	0.38	0.91	0.66	n/a	0.25	0.61	0.50	n/a	0.50	0.67	0.86

854 0 0 0 0 0 0 0 0 1068 0.80 n/a n/a n/a n/a

n/a

0 0

0 0 0 0

0 0

> n/a n/a

n/a

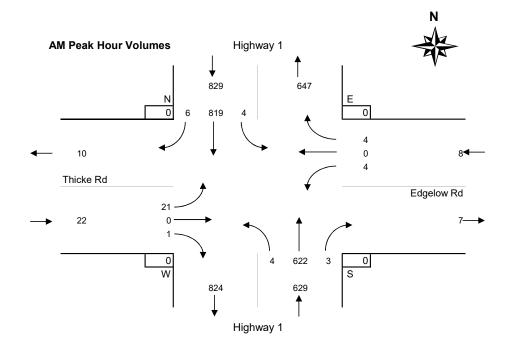
854

1068

0.80

Peak hour of conflicting volumes for the intersection

Pk Hr	4	819	6	829	4	622	3	629	21	0	1	22	4	0	4	8	1488
15x4	8	1008	12	1020	8	680	8	692	32	0	4	36	8	0	8	12	1740
PHF	0.50	0.81	0.50	0.81	0.50	0.91	0.38	0.91	0.66	n/a	0.25	0.61	0.50	n/a	0.50	0.67	0.86



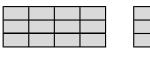


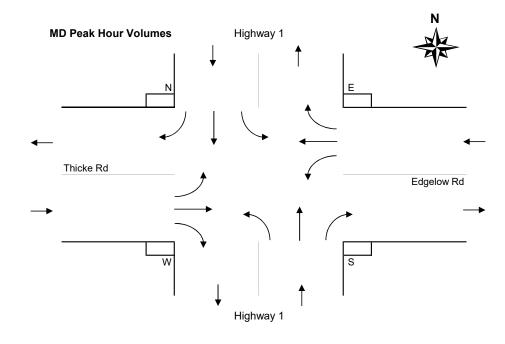
MD Peak Period All Vehicles Combined

Highway 1 @ Edgelow Road Thursday, August 9, 2018

Time		High	way 1			High	way 1				ke Rd			Edgel	ow Rd									_	
Period	NO	RTH	Appro	ach	SO		Appro		W	EST A	Approa	ach	E/	AST A	pproa	ich	Total V	olume	ak		Cross	swalks	6	Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Total V 15-min	Hour	Ре	Ν	S	W	Е	15 min	Hr
n/a																									
n/a																									
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							-	-		-	_														
Total																									
Avg Hr																									
Peak h	.	fthai	ntoro	ootio	•																				
Peak II Pk Hr	ouro	i uie	niers	ection	1													*	I					1 r	
15x4																		+						{ }	
PHF																		т						4 1	
FITE																		l	I					1 L	
Peak h	our o	f con	flictin	g volu	umes	for th	e inte	rsecti	ion																
Pk Hr																		*] [

PK Hr									*
15x4									+
PHF									







PM Peak Period All Vehicles Combined

Highway 1 @ Edgelow Road Thursday, August 9, 2018

Time		High				High	vay 1				ke Rd				ow Rd										
Period	NO	RTH /	Appro	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total V	olume	ak	(Cross	walks		Cor	nflict
Begins	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	15-min	Hour	Pe	N	S	W	Е	15 min	Hr
15:00	2	232	7	241	2	273	2	277	6	2	1	9	0	0	1	1	528			0	0	0	0	284	
15:15	0	256	3	259	4	289	0	293	12	0	2	14	0	0	2	2	568			0	0	0	0	303	
15:30	0	257	3	260	6	273	0	279	7	1	1	9	0	0	0	0	548			0	0	0	0	280	
15:45	2	286	0	288	4	249	0	253	12	0	0	12	1	0	2	3	556	2200		0	0	0	0	304	1171
16:00	0	245	4	249	3	299	0	302	22	0	0	22	0	0	0	0	573	2245	*	0	0	0	0	321	1208
16:15	3	231	5	239	7	287	1	295	13	1	1	15	0	0	1	1	550	2227	*	0	0	0	0	305	1210
16:30	0	246	3	249	1	334	1	336	9	0	0	9	1	1	2	4	598	2277	*	0	0	0	0	347	1277
16:45	0	253	1	254	6	340	1	347	9	0	1	10	0	0	1	1	612	2333	+	0	0	0	0	351	1324
17:00	3	240	2	245	1	288	0	289	9	0	0	9	0	0	3	3	546	2306		0	0	0	0	303	1306
17:15	0	260	0	260	1	259	2	262	8	0	0	8	0	0	0	0	530	2286		0	0	0	0	269	1270
17:30	1	256	0	257	5	235	0	240	5	0	1	6	0	0	0	0	503	2191		0	0	0	0	266	1189
17:45	1	222	0	223	2	243	1	246	3	0	0	3	0	0	1	1	473	2052		0	0	0	0	249	1087
n/a																									
n/a																									
n/a																									
n/a																									
n/a																									
n/a																									
n/a																									
n/a																									
Total	12	2984	28	3024	42	3369	8	3419	115	4	7	126	2	1	13	16	6585			0	0	0	0	l	3518
Avg Hr	4	995	9	1008	14	1123	3	1140	38	1	2	42	1	0	4	5	2195			0	0	0	0		

Peak hour of the intersection

Pk Hr	3	975	13	991	17	1260	3	1280	53	1	2	56	1	1	4	6	2333
15x4	12	1012	20	1016	28	1360	4	1388	88	4	4	88	4	4	8	16	2448
PHF	0.25	0.96	0.65	0.98	0.61	0.93	0.75	0.92	0.60	0.25	0.50	0.64	0.25	0.25	0.50	0.38	0.95

0	0	0	0	1324	
0	0	0	0	1476	
n/a	n/a	n/a	n/a	0.90	

n/a

0 0

1324

1476

0.90

0 0 0 0

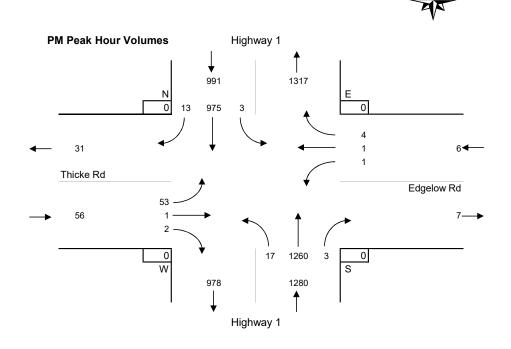
0 0

n/a n/a

n/a

Peak hour of conflicting volumes for the intersection

Pk Hr	3	975	13	991	17	1260	3	1280	53	1	2	56	1	1	4	6	2333
15x4	12	1012	20	1016	28	1360	4	1388	88	4	4	88	4	4	8	16	2448
PHF	0.25	0.96	0.65	0.98	0.61	0.93	0.75	0.92	0.60	0.25	0.50	0.64	0.25	0.25	0.50	0.38	0.95





Entire Survey Period

			High	way 1			High	way 1			Thick	ke Rd			Edgel	ow Ro			 				
		NO	RTH	Appro	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ich	Total		0	Cross	walks	,
		Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume		N	S	W	
-	Гotal	16	4576	47	4639	55	4615	11	4681	147	5	8	160	6	1	19	26	9506		0	0	0	
/	Avg Hr	3	915	9	928	11	923	2	936	29	1	2	32	1	0	4	5	1901		0	0	0	

AM Peak Period

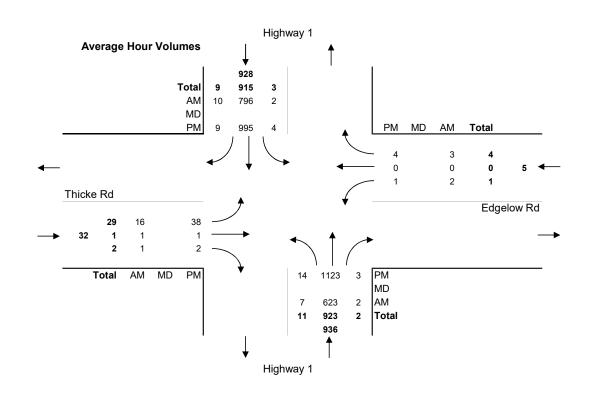
		High	way 1			High	way 1			Thick	ke Rd			Edgel	ow Ro					
	NO	RTH	Appro	ach	SO	UTH /	Appro	ach	W	EST A	Approa	ach	E	AST A	pproa	ich	Total		Cross	wa
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	N	S	V ا
Period	4	1592	19	1615	13	1246	3	1262	32	1	1	34	4	0	6	10	2921	0	0	
Avg Hr	2	796	10	808	7	623	2	631	16	1	1	17	2	0	3	5	1461	0	0	

MD Peak Period

		High	way 1			High	way 1			Thick	ke Rd			Edgel	ow Rd			 				
	NO	RTH	Appro	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total		C	Cross	walks	,
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	1	1	S	W	Е
Total																						
Avg Hr																						

PM Peak Period

		High	way 1			High	way 1			Thick	ke Rd			Edgel	ow Rd						
	NO	RTH /	Approa	ach	SO	UTH /	Approa	ach	W	EST A	Approa	ach	E/	AST A	pproa	ch	Total		Cross	walks	;
Totals	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Left	Thru	Right	Total	Volume	N	S	W	Е
Total	12	2984	28	3024	42	3369	8	3419	115	4	7	126	2	1	13	16	6585	0	0	0	0
Avg Hr	4	995	9	1008	14	1123	3	1140	38	1	2	42	1	0	4	5	2195	0	0	0	0





Highway 1 @ Edgelow Road Thursday, August 9, 2018

W Е 0 0

0 0

E 0 0

0 0

5 Hours

Hours

APPENDIX B

MOTI TRAFFIC SIGNAL WARRANT ANALYSIS





1 - *BINNIE* File No. 18-0628-05

Appendix B - MOTI Traffic Signal Warrant Analysis

Intersection Lo	cation.	т	of Loducersid	Traffic Count E	Grouhel Road		101164 2010
District		Iown	of Ladysmith		Data:		Igust 2018
Completed by:			ML	Warrant Date:		Sept	ember 20
Data Require	ments					-	
Adjacent Land I		e):			Urban	Rural	
Population of B	•			8,600			
Signal Correcta	ble Accident F	requency (acc/)	/):	4.6			
Major Street:		Hi	ghway I	Minor Street:		G	rouhel Rd
Number of Inco	oming Lanes or	n Each Approac	h:	Number of Inco	oming Lanes c	on Each Appro	bach:
	Northbound:		2		Eastbound:		I.
	Southbound:		2		Westbound:		0
Existing Traffic	Volumes (by A	(pproach)		Existing Traffic	Volumes (by	Approach)	
7 hours traffic vo				7 hours traffic ve		•••	
		- ,	Total of			- •	Higher o
Time Period	Southbound	Northbound	Both	Time Period	Eastbound	Westbound	Each
7am to 8am	782	976	1758	7am to 8am	54		54
8am to 9am	941	918	1859	8am to 9am	46		46
3pm to 4pm	1373	1316	2689	3pm to 4pm	38		38
4pm to 5pm	1397	I 448	2845	4pm to 5pm	49		49
5pm to 6pm	1459	1279	2738	5pm to 6pm	35		35
Highest of 4 con	secutive hours o	on an average da	v *	Highest of 4 con	secutive hours	on an average	dav*
0			, Total of	8			Higher o
Time Period	Southbound	Northbound	Both	Time Period	Eastbound	Westbound	Each
8am to 9am	941	918	1859	8am to 9am	46		46
3pm to 4pm	1373	1316	2689	3pm to 4pm	38		38
4pm to 5pm	1397	1448	2845	4pm to 5pm	49		49
5pm to 6pm	1459	1279	2738	5pm to 6pm	35		35
Peak hour traffic	volumes on an	average dav		Peak hour traffic	volumes on a	n average dav	
		8 /	Total of			0 /	Higher o
Time Period	Southbound	Northbound	Both	Time Period	Eastbound	Westbound	Each
415pm to	1464	1515	2979	415pm to	50		50
515pm				515pm			
5 Year Projecte	d Traffic Volu	mas (by Approx	ych)	5 Year Projecte	d Traffic Vel	imes (by App	roach)
7 hours traffic vo		.,		7 hours traffic ve			Jacij
Southbound 5-v	year Growth F	actor ⁺ =	1.10	Eastbound 5-ye	ar Growth Fa	ctor ⁺ =	1.10
,	year Growth I	- +	1.10	Westbound 5-y		+ _	1.10



2 - BINNIE File No. 18-0628-05

Appendix B - MOTI Traffic Signal Warrant Analysis

Intersection Lo	cation:				Highway I and G	Grouhel Road		
District		Town	of Ladysmith	Ì	Traffic Count D	ata:	Au	gust 2018
Completed by:			ML		Warrant Date:		Sept	ember 2018
			Total of					Higher of
Time Period		Northbound	Both		Time Period		Westbound	Each
7am to 8am	863	1078	1941		7am to 8am	60		60
8am to 9am	1039	1014	2052		8am to 9am	51		51
3pm to 4pm	1516	1453	2969		3pm to 4pm	42		42
4pm to 5pm	1542	1599	3141		4pm to 5pm	54		54
5pm to 6pm	1611	1412	3023		5pm to 6pm	39		39
Highest of 4 con	secutive hours o	on an average da			Highest of 4 cons	secutive hours	on an average	-
			Total of					Higher of
Time Period		Northbound	Both		Time Period		Westbound	Each
8am to 9am	1039	1014	2052		8am to 9am	51		51
3pm to 4pm	1516	1453	2969		3pm to 4pm	42		42
4pm to 5pm	1542	1599	3141		4pm to 5pm	54		54
5pm to 6pm	1611	1412	3023		5pm to 6pm	39		39
Peak hour traffic	volumes on an	average day			Peak hour traffic	volumes on ar	n average day	
			Total of					Higher of
Time Period		Northbound	Both		Time Period		Westbound	Each
415pm to	1616	1673	3289		415pm to	55		55
515pm					515pm			
Major Route:		Yes	No		Major Route:		Yes	No
Posted or 85th P	ercentile Speed	(km/hr)**:	110		Existing Peak He	our Delay (ve	eh-hr) ⁺⁺ :	
Distance to Ne	arest Signal (m	ı):	540					
						Eastbound	_	6.02
Traffic Directio	n:	2-way	l-way			Westbound		0.00

* Note: The Ministry standard is to use the highest 4 consecutive hours of an average day, however, since the longest consecutive hours from thetraffic survey was 3 hours, the highest of 3 consecutive hours will be used in the analysis.

** Actual observed speed is higher than the poted speed limit.

+ Growth factors for the major route were calculated by using historical AADT data, growth factors for the minor route were calculated by taking the average of the growth factors along both directions of the major route.

++ Peak hour delays were calculated using the Synchro software.

Questions which we do not have information on are left blank, and questions which are "Not Applicable" are crossed-out.



WARRANT NO.I MINIMUM VEHICULAR VOLUME

		Larg	ge Urban Areas	s (> 10,000 рори	lation)		oan Areas oopulation)
			Posted or 85t	h Percentile Spe	ed		
		=< 70	km/hr	> 70	km/hr	Pools 7 Hours	Volume (vph)
	oming Lanes on roach	Peak 7 Hour	Volume (vph)	Peak 7 Hour	Volume (vph)	reak / Hour	volume (vpn)
Major	Minor	Major	Minor	Major	Minor	Major	Minor
I	I	500	150	350	105	350	105
2 or more	I	600	150	420	105	420	105
2 or more	2 or more	600	200	420	140	420	140
I	2 or more	500	200	350	140	350	140

E	xisting Scenari	io to be Conside	red
Number of In	coming Lanes		
on Ap	proach	Minimum	Volumes
Major	Minor	Major	Minor
2 or more		420	105

Existing Traffic Volumes (by Approach)

7 hours traffic vol	lume on an average day			
	Total of Both Major	Higher tha		
Time Period	Approaches	Minimum?		
7am to 8am	1758	Yes		
8am to 9am	1859	Yes		
3pm to 4pm	2689	Yes		
4pm to 5pm	2845	Yes		
5pm to 6pm	2738	Yes		

Existing Traffic Volumes (by Approach)

7 hours traffic volume on an average day

		- ,	
an	Time Period	Higher of Each Minor	Higher than
n?		Approaches	Minimum?
	7am to 8am	54	No
	8am to 9am	46	No
	3pm to 4pm	38	No
	4pm to 5pm	49	No
	3pm to 4pm 4pm to 5pm 5pm to 6pm	35	No

Warrant Satisfied?

No

Yes

Explanation: The warrant is not satisfied. No hours of minor street traffic volume exceeds the minimum vehicular volume criteria.



WARRANT NO.2 Interruption of Continuous Traffic

		Larg	Large Urban Areas (> 10,000 population) Small Urban Area (<10,000 population)				
		Posted or 85th Percentile Speed					
		=< 70	km/hr	> 70 km/hr		Peels 7 Hours Volume (uph)	
Number of Incoming Lanes on Approach		Peak 7 Hour	r Volume (vph) Peak 7 Hour		Volume (vph)	Peak 7 Hour Volume (vph)	
Major	Minor	Major	Minor	Major	Minor	Major	Minor
I	I	750	75	525	50	525	50
2 or more	I	900 75		630	50	630	50
2 or more	2 or more	900	100	630	70	630	70
I	2 or more	750	100	525	70	525	70

Existing Scenario to be Considered					
Number of In	coming Lanes				
on Ap	proach	Minimum Volumes			
Major	Minor	Major	Minor		
2 or more		630	50		

Existing Traffic Volumes (by Approach) Existing Traffic Volumes (by Approach) 8 hours traffic volume on an average day 8 hours traffic volume on an average day Total of Both Major Higher than Time Period Higher of Each Minor Higher than **Time Period Approaches** Minimum? **Approaches** Minimum? 7am to 8am 1758 Yes 7am to 8am 54 Yes 8am to 9am 1859 8am to 9am Yes 46 No 3pm to 4pm 2689 Yes 3pm to 4pm 38 No 4pm to 5pm 49 2845 Yes 4pm to 5pm No 5pm to 6pm 2738 Yes 5pm to 6pm 35 No

Warrant Satisfied?

No

Yes

Explanation: The warrant is not satisfied. 4 hours of minor street traffic volume do not exceed the minimum vehicular volume criteria.

5 - BINNIE File No. 18-0628-05



Appendix B - MOTI Traffic Signal Warrant Analysis

WARRANT NO.3 Progressive Movement		
1) Is the distance to the nearest signal greater than or equal to 300m?	Yes	No
One Way		
Are the adjacent signals so far apart that they do not provide a necessary	×~	Ho
degree of vehicle platooning and speed control?		
<u>Two Way</u>		
Do the adjacent signals constitute a progressive system?	Yes	No
Are the adjacent signals so far apart that they do not provide a necessary	Yes	No
degree of vehicle platooning and speed control?		-
Warrant Satisfied? Yes No		
Explanation: The adjacent signals do not constitute a progressive system.		
WARRANT NO.4 Accident Experience (based on ICBC Claims	s Data)	
1) Have five or more reported accidents of types susceptible to correction	Yes	No
by traffic signals occurred within a 12 month period, with each accident		L
involving personal injury or damage exceeding \$1000?		
2) Have adequate trials of less restrictive remedies with satisfactory	Yes	No
observance and enforcement failed to reduce the accident frequency?		
3) Will the installation of a signal allow progressive traffic flow?	Yes	No
Warrant Satisfied? Yes No		
Explanation: - Previous 5 years (2012 to 2017): 4 accidents total as per N	10TI data	
WARRANT NO.5 System Warrant		
I) Are both the major and minor streets "Major Routes"?	Yes	No
2) Does the total Peak Hour Volume over all approaches equal or exceed 1000 vph?	>*~<	
3) Are one or more of Warrants 1,2,6,7 and 9 satisfied using Projected 5 Year Volumes?	>**	
4) Does the Peak 5 Hour Weekend Volume equal or exceed 1000 vph?	>	
Warrant Satisfied? Yes No		
Explanation: The warrant is not satisfied because Highway I is the only m	ajor route.	



6 - **BINNIE** File No. 18-0628-05 Appendix B - MOTI Traffic Signal Warrant Analysis

WARRANT NO.6 Combination Warrant

Yes

Small Urban Areas

No

1) Have other measures been tried which cause less delay and invonvenience to traffic than traffic signals?

		Larg	e Urban Areas	(<10,000 population)				
			Posted or 85t					
		=< 70	km/hr	> 70	km/hr			
Number of Inc	oming Lanes on					1		
Appr	Approach		Peak 7 Hour Volume (vph)		Peak 7 Hour Volume (vph)		Peak 7 Hour Volume (vph)	
Major	Minor	Major	Minor	Major	Minor	Major	Minor	
I	I	600	120	420	85	420	85	
2 or more	I	720	120	500	85	500	85	
2 or more	2 or more	720	160	500	110	500	110	
I	2 or more	600	160	420	110	420	110	

Existing Scenario to be Considered					
Number of In	coming Lanes	6			
on Ap	proach	Minimum Volumes			
Major	Minor	Major	Minor		
2 or more	I	500	85		

Existing Traffic \	/olumes (by Approach)		Existing Traffic Volumes (by Approach)				
7 hours traffic vol	ume on an average day		7 hours traffic volume on an average day				
	Total of Both Major Higher than			Higher of Each Minor	Higher than		
Time Period	Approaches	Minimum?		Approaches	Minimum?		
7am to 8am	1758	Yes	7am to 8am	54	No		
8am to 9am	1859	Yes	8am to 9am	46	No		
3pm to 4pm	2689	Yes	3pm to 4pm	38	No		
4pm to 5pm	2845	Yes	4pm to 5pm	49	No		
5pm to 6pm	2738	Yes	5pm to 6pm	35	No		
			-				

Warrant Satisfied? Yes

No

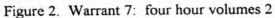
Explanation: The warrant is not satisfied. All hours of minor street traffic volume does not exceed the minimum vehicular volume criteria.



7 - **BINNIE** File No. 18-0628-05 Appendix B - MOTI Traffic Signal Warrant Analysis

WARRANT NO.7 Four Hour Volumes						
	Posted or 85th Percentile Speed					
	=< 70 km/hr	> 70 km/hr				
Rural	Figure I	Figure 2				
Large Urban	Figure 1	Figure 2				
(>10,000 pop.)	Figure I	Figure 2				
Small Urban	F : 0	5: 0				
(<10,000 pop.)	Figure 2	Figure 2				

	Ex	Existing Scenario to be Considered									
	Location Type			Figure							
	Small l	Jrban (<	10,000 ро	р.)	Figure 2						
lighest of 4 cons	ecutive h	ours on c	in average	day		Hiş	hest of 4 co	onsecutive	hours on	n an average day	/
											Higher o
Time Period	South	bound	Northbo	und T	otal of Bo	th T	ime Period	East	bound	Westbound	Each
8am to 9am	94	41	918		1859	8	am to 9am	1	46	0	46
3pm to 4pm	13	73	1316		2689	3	pm to 4pm	n :	38	0	38
4pm to 5pm	13	97	1448		2845	4	pm to 5pm	n -	49	0	49
5pm to 6pm	14	59	1279		2738	5	pm to <mark>6</mark> pm	n :	35	0	35
Minor Street - High Volume Approach Volume (vph)	300	/ /	1				1 lane or	e lanes on ma n minor street ARRA	NTEL	>	
Minor Street -	100	NO	T WARI			ne is 80 v	wh far minor stre	1 lane or	n major stro n minor stro more lanes	eet	
					for minor stree			1	1		
	0	30	00 40	00	500	600	700	800	90	00	
				Ма		otal of E dume (v	oth Approact ph)	nes			



Warrant Satisfied? Yes No Explanation: The warrant is not satisfied because none of the 4 consecutive hours exceed or equal the appropriate threshold.



WARRANT NO.8 Peak Hour Delay

		oming Lanes on Approach with k Hour Delay
		2 or more
Minimum Peak Hour Delay (veh hr)	4	5
Minimum Peak Hour Traffic (vph)	100	150

	Minimum total Peak Hour
Number of Intersection	Traffic for All Approaches
Approaches	Combined (vph)
3	650
4	800

Existing Scenario to be Considered			
Minimum Peak Hour Delay			
(veh-hr)	4		
Minimum Peak Hour Traffic			
(vph)	100		
Minimum total Peak Hour			
Traffic for All Approaches			
Combined (vph)	650		

Peak hour traffic volumes on an average day				Peak hour traffic volumes on an average day			
							Higher of
Time Period	Southbound	Northbound	Total of Both	Time Period	Eastbound	Westbound	Each
415pm to	1464	1515	2979	415pm to	50	0	50
515pm				515pm			
				Existing Peak Ho	our Delay (veh-	hr):	
					Eastbound:		6.02
					Westbound:	:	0.00
\sim	arrant Satisfied?	Yes	No				
	Explanation:	The warrant is	s not satisfied be	cause the existin	g peak hour		
traffic for the minor approach				does not exceed	100 vehicles		
		per hour					



WARRANT NO.9 Peak Hour Volumes

	Large Urban Areas (> 10000 population)					
	Posted or 85th Percentile Speed					
Location Type	=< 70 km/hr	> 70 km/hr				
Rural	Figure 3	Figure 4				
Large Urban						
(>10,000 pop.)	Figure 3	Figure 4				
Small Urban						
(<10,000 pop.)	Figure 4	Figure 4				

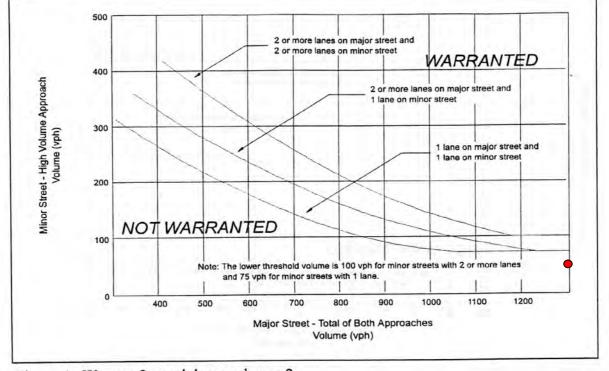
Existing Scenario to be Considered				
Location Type	Figure			
,1	rigure			
Small Urban (<10,000 pop.)	Figure 4			

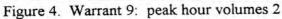
Peak hour traffic volumes on an average day

Peak hour traffic volumes on an average day

Lichan of

							Higher of
Time Period	Southbound	Northbound	Total of Both	Time Period	Eastbound	Westbound	Each
415pm to	1464	1515	2979	415pm to	50	0	50
515pm				515pm			





Warrant Satisfied? Yes No Explanation: The warrant is not satisfied because the peak hour volume does not exceed or equal the required threshold.



Warrant

I) Minimum Vehicular Volume	Satisfied	× Not Satisfied
2) Interruption of Continuous Traffic	Satisfied	× Not Satisfied
3) Progressive Movement	Satisfied	× Not Satisfied
4) Accident Experience	Satisfied	× Not Satisfied
5) System Warrant	Satisfied	× Not Satisfied
6) Combination Warrant	Satisfied	× Not Satisfied
7) Four Hour Volume	Satisfied	× Not Satisfied
8) Peak Hour Delay	Satisfied	× Not Satisfied
9) Peak Hour Volume	Satisfied	× Not Satisfied

Comments:

- This intersection does not warrant the installation of a traffic signal.



Appendix B - MOTI Traffic Signal Warrant Analysis

Intersection Loc	cation:			Highway I and S Davis St			
District Town of Ladysmith			Traffic Count D	Traffic Count Data:			
Completed by: ML			ML	Warrant Date:	Warrant Date:		
Data Require		`					
Adjacent Land L		e):		9 (00	Urban	Rural	
Population of Bu	-	roquoney (accl		8,600			
Signal Correctal	Die Accident F	requency (acc/	y):	0.2		_	
Major Street:		Hi	ighway l	Minor Street:		[Davis St.
, Number of Inco	ming Lanes or			Number of Inco	oming Lanes o	on Each Appro	ach:
	Northbound:		2		Eastbound:		I
	Southbound:		2		Westbound:		I
		-					
Existing Traffic				Existing Traffic		•••	
7 hours traffic vo	lume on an ave	rage day		7 hours traffic vo	lume on an av	verage day	
	_		Total of				Higher of
Time Period		Northbound	Both	Time Period		Westbound	Each
7am to 8am	792	625	1417	7am to 8am	68	3	68
8am to 9am	787	655	1442	8am to 9am	45	l	45
3pm to 4pm	1091	1133	2224	3pm to 4pm	44	1	44
4pm to 5pm	1034	1318	2352	4pm to 5pm	49	2	49
5pm to 6pm	1033	1072	2105	5pm to 6pm	35	2	35
Highest of 1 cons	acutiva haura a	n an avorado d	 *	Highest of 1 con	acutiva haura	on an avorago	dau*
Highest of 4 cons	eculive nours o	n an average ad	Total of	Highest of 4 con	seculive nours	on an average	Higher of
Time Period	Southbound	Northbound	Both	Time Period	Fastbound	Westbound	Each
8am to 9am	787	655	1442	8am to 9am	45	I	45
3pm to 4pm	1091	1133	2224	3pm to 4pm	44	· ·	44
4pm to 5pm	1034	1318	2352	4pm to 5pm	49	2	49
5pm to 6pm	1033	1072	2105	5pm to 6pm	35	2	35
Peak hour traffic	volumes on an	average day		Peak hour traffic	volumes on a	n average day	
			Total of				Higher of
Time Period	Southbound	Northbound	Both	Time Period	Eastbound	Westbound	Each
415pm to	1037	1325	2362	415pm to	51	2	51
515pm				515pm			
5 Year Projecte		() 11	ach)	5 Year Projecte		.,	·oach)
7 hours traffic vo	lume on an ave	rage day		7 hours traffic vo	lume on an av	erage day	
Caralle 15		+ _		Fred 15		+_	
Southbound 5-y			1.10	Eastbound 5-ye			1.10
Northbound 5-	/ear Growth F	actor =	1.10	Westbound 5-year Growth Factor ⁺ = 1.10			



12 - **BINNIE** File No. 18-0628-05

Appendix B - MOTI Traffic Signal Warrant Analysis

Intersection Lo	cation:		Highway I an	d S Davis St				
District		Town	of Ladysmith	Traffic Count D	Traffic Count Data:		August 2018	
Completed by:			ML	Warrant Date:		Sept	ember 2	
			Total of				Higher	
Time Period	Southbound	Northbound	Both	Time Period	Eastbound	Westbound	Each	
7am to 8am	874	690	1564	7am to 8am	75	3	75	
8am to 9am	869	723	1592	8am to 9am	50	I.	50	
3pm to 4pm	1205	1251	2455	3pm to 4pm	49	l I	49	
4pm to 5pm	1142	1455	2597	4pm to 5pm	54	2	54	
5pm to 6pm	1141	1184	2324	5pm to 6pm	39	2	39	
Highest of 4 con	secutive hours o	n an average da	v	Highest of 4 con	secutive hours	on an average	dav	
8 1		8	, Total of	5 1		0	, Higher	
Time Period	Southbound	Northbound	Both	Time Period	Eastbound	Westbound	Each	
7am to 8am	869	723	1592	8am to 9am	50	l I	50	
8am to 9am	1205	1251	2455	3pm to 4pm	49	l I	49	
4pm to 5pm	1142	1455	2597	4pm to 5pm	54	2	54	
5pm to 6pm	1141	1184	2324	5pm to 6pm	39	2	39	
Peak hour traffic	volumes on an	average dav		Peak hour traffic	volumes on a	n average dav		
		8 /	Total of			8 /	Higher	
Time Period	Southbound	Northbound	Both	Time Period	Eastbound	Westbound	Each	
415pm to	1145	1463	2608	415pm to	56	2	56	
515pm				515pm				
Major Route:		Yes	No	Major Route:		Yes	No	
Posted or 85th P	-	· · · ·	110	Existing Peak H	lour Delay (ve	eh-hr) '' :		
Distance to Ne	arest Signal (m):	1034					
					Eastbound		0.76	
Traffic Directio	n:	2-way	l-way		Westbound		0.10	

* Note: The Ministry standard is to use the highest 4 consecutive hours of an average day, however, since the longest consecutive hours from the traffic survey was 3 hours, the highest of 3 consecutive hours will be used in the analysis.

** Actual observed speed is higher than the poted speed limit.

+ Growth factors for the major route were calculated by using historical AADT data, growth factors for the minor route were calculated by taking the average of the growth factors along both directions of the major route.

++ Peak hour delays were calculated using the Synchro software.

Questions which we do not have information on are left blank, and questions which are "Not Applicable" are crossed-out.



WARRANT NO.I MINIMUM VEHICULAR VOLUME

		Large Urban Areas (> 10,000 population)				Small Urban Areas (<10,000 population)	
			Posted or 85t	h Percentile Spe	ed		
		=< 70 km/hr		> 70 km/hr		Deels 7 Llaure Valumea (un h)	
Number of Incoming Lanes on Approach		Peak 7 Hour Volume (vph)		Peak 7 Hour Volume (vph)		Peak 7 Hour Volume (vph)	
Major	Minor	Major	Minor	Major	Minor	Major	Minor
I	I	500	150	350	105	350	105
2 or more	I	600	150	420	105	420	105
2 or more	2 or more	600	200	420	140	420	140
I	2 or more	500	200	350	140	350	140

Existing Scenario to be Considered						
Number of In	coming Lanes					
on Ap	proach	Minimum Volumes				
Major	Minor	Major	Minor			
2	I	420	105			

Existing Traffic V	olumes (by Approach)		Existing Traffic Volumes (by Approach)				
7 hours traffic volume on an average day			7 hours traffic volume on an average day				
	Total of Both Major	Higher than	Time Period	Higher of Each Minor	Higher than		
Time Period	Approaches	Minimum?		Approaches	Minimum?		
7am to 8am	1417	Yes	7am to 8am	68	No		
8am to 9am	1442	Yes	8am to 9am	45	No		
2pm to 3pm	2224	Yes	2pm to 3pm	44	No		
3pm to 4pm	2352	Yes	3pm to 4pm	49	No		
4pm to 5pm	2105	Yes	4pm to 5pm	35	No		
			-				

Warrant Satisfied?

No

Yes

Explanation: The warrant is not satisfied. The minor approach does not exceed the minimum volume criteria.



		Larg	ge Urban Areas	Small Urban Areas (<10,000 population)			
			Posted or 85t	h Percentile Spe	ed		
		=< 70 km/hr		> 70 km/hr		Peak 7 Hour Volume (vph)	
Number of Incoming Lanes on Approach		Peak 7 Hour Volume (vph)		Peak 7 Hour Volume (vph)			
Major	Minor	Major	Minor	Major	Minor	Major	Minor
I	I	750	75	525	50	525	50
2 or more	I	900	75	630	50	630	50
2 or more	2 or more	900	100	630	70	630	70
I	2 or more	750	100	525	70	525	70

WARRANT NO.2 Interruption of Continuous Traffic

Existing Scenario to be Considered						
Number of In	coming Lanes					
on Ap	proach	Minimum Volumes				
Major	Minor	Major	Minor			
2	I	630	50			

Existing Traffic \	olumes (by Approach)		Existing Traffic Volumes (by Approach)				
8 hours traffic volume on an average day			8 hours traffic volume on an average day				
	Total of Both Major	Higher than	Time Period	Higher of Each Minor	Higher than		
Time Period	Approaches	Minimum?		Approaches	Minimum?		
7am to 8am	1417	Yes	7am to 8am	68	Yes		
8am to 9am	1442	Yes	8am to 9am	45	No		
3pm to 4pm	2224	Yes	3pm to 4pm	44	No		
4pm to 5pm	2352	Yes	4pm to 5pm	49	No		
5pm to 6pm	2105	Yes	5pm to 6pm	35	No		

Warrant Satisfied?

No

Yes

Explanation: The warrant is not satisfied. Most hours of minor street traffic volume do not exceed the minimum vehicular volume criteria.

BINNIE	Appendix B - MOTI Traffic Signal Warrant An
WARRANT NO.3 Progress	ssive Movement
I) Is the distance to the nearest signal greater than or equal to 300r	
<u>One Way</u> Are the adjacent signals so far apart that they do not provide a neces degree of vehicle platooning and speed control?	essary
<u>Two Way</u> Do the adjacent signals constitute a progressive system?	Yes No
Are the adjacent signals so far apart that they do not provide a neces degree of vehicle platooning and speed control?	essary Yes No
Warrant Satisfied? Yes No Explanation: The adjacent signals do not constitu	ute a progressive system.
WARRANT NO.4 Accident Experience ((based on ICBC Claims Data)
 Have five or more reported accidents of types susceptible to cor by traffic signals occurred within a 12 month period, with each accide involving personal injury or damage exceeding \$1000? 	
2) Have adequate trials of less restrictive remedies with satisfactory observance and enforcement failed to reduce the accident frequency	
3) Will the installation of a signal allow progressive traffic flow?	Xac
Warrant Satisfied? Yes No Explanation: - Previous 5 years (2012 to 2016):	I accidents total based on MOTI data
WARRANT NO.5 Syste	em Warrant
I) Are both the major and minor streets "Major Routes"?	Yes No
2) Does the total Peak Hour Volume over all approaches equal or e 1000 vph?	exceed Xec
3) Are one or more of Warrants 1,2,6,7 and 9 satisfied using Project Year Volumes?	cted 5
4) Does the Peak 5 Hour Weekend Volume equal or exceed 1000 v	vph?
Warrant Satisfied? Yes No Explanation: The warrant is not satisfied because	e Highway I is the only major route.

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WARRANT NO.6 Combination Warrant

I) Have other measures been tried which cause less delay and

Yes

No

invonvenience to traffic than traffic signals?

						Small Urt	oan Areas
		Large Urban Areas (> 10,000 population)			(<10,000 population)		
			Posted or 85th Percentile Speed				
		=< 70	km/hr	> 70	km/hr	1	
Number of Inco	oming Lanes on			1			
Appr	oach	Peak 7 Hour	our Volume (vph) Peak 7 Hour Volume (vph)		Peak 7 Hour Volume (vpł		
Major	Minor	Major	Minor	Major	Minor	Major	Minor
I	I	600	120	420	85	420	85
2 or more	I	720	120	500	85	500	85
2 or more	2 or more	720	160	500	110	500	110
I	2 or more	600	160	420	110	420	110

Existing Scenario to be Considered				
Number of In	coming Lanes			
on Approach		Minimum	Volumes	
Major	Minor	Major	Minor	
2	I	500	85	

Existing Traffic \	sting Traffic Volumes (by Approach) Existing Traffic Volumes (by Approach)					
7 hours traffic vol	lume on an average day		7 hours traffic volume on an average day			
	Total of Both Major	Higher than	Time Period	Higher of Each Minor	Higher than	
Time Period	Approaches	Minimum?		Approaches	Minimum?	
7am to 8am	1417	Yes	7am to 8am	68	No	
8am to 9am	1442	Yes	8am to 9am	45	No	
2pm to 3pm	2224	Yes	2pm to 3pm	44	No	
3pm to 4pm	2352	Yes	3pm to 4pm	49	No	
4pm to 5pm	2105	Yes	4pm to 5pm	35	No	
			-			
V	/arrant Satisfied? Yes	No				

No

Explanation: The warrant is not satisfied. All hours of the minor route's traffic volume

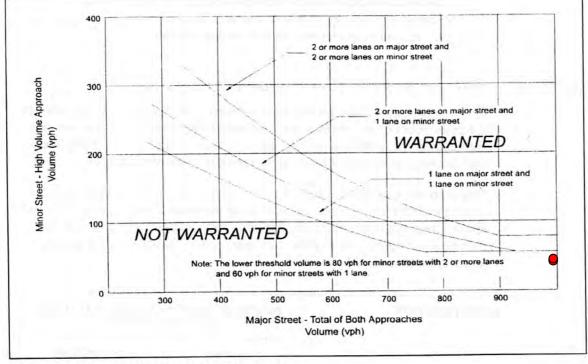
does not exceed the minimum volume criteria.

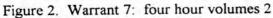


17 - **BINNIE** File No. 18-0628-05 Appendix B - MOTI Traffic Signal Warrant Analysis

WARRANT NO.7 Four Hour Volumes						
	Posted or 85th Percentile Speed					
	=< 70 km/hr	> 70 km/hr				
Rural	Figure I	Figure 2				
Large Urban	Figure 1	Figure 2				
(>10,000 pop.)	Figure I					
Small Urban	F igure 2	5:				
(<10,000 pop.)	Figure 2	Figure 2				

	Existing Scenario to be Considered						
	Locatio	n Type	Figure				
	Small Urban (<	<10,000 pop.)	Figure 2				
Highest of 4 consecutive hours on an average day		Highest of 4 cons	ecutive hours on	an average day			
							Higher of
Time Period	Southbound	Northbound	Total of Both	Time Period	Eastbound	Westbound	Each
8am to 9am	787	655	1442	8am to 9am	45	I	45
3pm to 4pm	1091	1133	2224	3pm to 4pm	44	I.	44
4pm to 5pm	1034	1318	2352	4pm to 5pm	49	2	49
5pm to 6pm	1033	1072	2105	5pm to 6pm	35	2	35





Warrant Satisfied? Yes No Explanation: The warrant is not satisfied because none of the 3 consecutive hours exceeds or equals the appropriate threshold.



WARRANT NO.8 Peak Hour Delay

	Number of Minor Street Incoming Lanes on Approach with Highest Peak Hour Delay				
		2 or more			
Minimum Peak Hour Delay (veh hr)	4	5			
Minimum Peak Hour Traffic (vph)	100	150			

	Minimum total Peak Hour
Number of Intersection	Traffic for All Approaches
Approaches	Combined (vph)
3	650
4	800

Existing Scenario to be Considered				
Minimum Peak Hour Delay				
(veh-hr)	4			
Minimum Peak Hour Traffic				
(vph)	100			
Minimum total Peak Hour				
Traffic for All Approaches				
Combined (vph)	800			

Peak hour traffic	volumes on an av	erage day		Peak hour traffic	volumes on an a	verage day	
							Higher of
Time Period	Southbound	Northbound	Total of Both	Time Period	Eastbound	Westbound	Each
415pm to	1037	1325	2362	415pm to	51	2	51
5 I 5pm				515pm			
				Existing Peak Ho	our Delay (veh-	hr):	
					Eastbound	:	0.76
					Westbound	:	0.10
		_		_			
V	/arrant Satisfied?	Yes	No				
	Explanation:	The warrant is	s not satisfied be	cause the existin	g peak hour		

delay for the minor approach does not exceed 4 veh-hr.



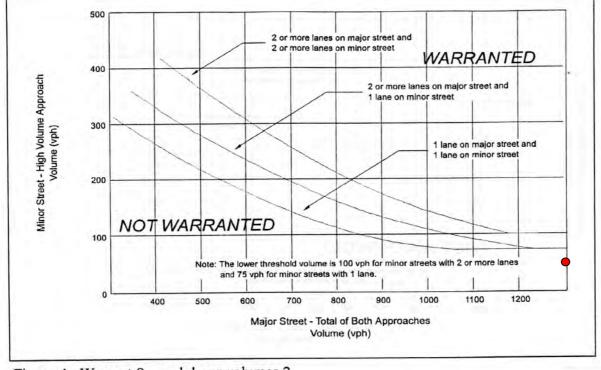
WARRANT NO.9 Peak Hour Volumes							
	Large Urban Areas (> 10000 population)						
	Posted or 85th	Percentile Speed					
Location Type	=< 70 km/hr	> 70 km/hr					
Rural	Figure 3	Figure 4					
Large Urban							
(>10,000 pop.)	Figure 3	Figure 4					
Small Urban							
(<10,000 рор.)	Figure 4	Figure 4					

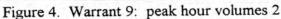
Existing Scenario to be Considered				
Location Type Figure				
Small Urban (<10,000 pop.)	Figure 4			

Peak hour traffic volumes on an average day

Peak hour traffic volumes on an average day

							Higher of
Time Period	Southbound	Northbound	Total of Both	Time Period	Eastbound	Westbound	Each
415pm to	1037	1325	2362	415pm to	51	2	51
515pm				515pm			





Warrant Satisfied?

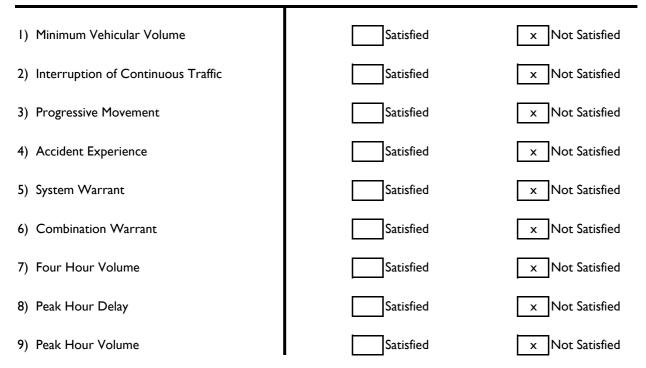
No

Explanation: The warrant is not satisfied because the peak hour volume does not exceed or equal the required threshold.

Yes



Warrant



Comments:

- This intersection does not warrant the installation of a traffic signal.



MINISTRY OF TRANSPORTATION AND INFRASTRUCTURE LEFT TURN WARRANT CALCULATIONS

WARRANT CALCULATED BY:	Max Leung	2019
DATE:	9/13/2018	
INPUT		

INTERSECTION DESCRIPTION			
INTERSECTION	=	Highway 1 & Transfer Beach Bly	vd/ Roberts St
N/S ROUTE	=	Highway 1	
DIRECTION	=	SBL	
TRAFFIC COUNT	=	9-Aug-18	
TIMING SHEET	=	N/A	

INTERSECTION GEOMETRICS		
# OF OPPOSING THRU LANES =	2	
SPEED LIMIT ON MAJOR STREET =	70	(km/hr)

TRAFFIC VOLUME INPUT				
ACTUAL VOLUME-AM (E/B LT)	=	21 (vph)		
ACTUAL VOLUME-NOON (E/B LT)	=	N/A (vph)		
ACTUAL VOLUME-PM (E/B LT)	=	29 (vph)		
OPPOSING VOLUME-AM (W/B Thru)	=	739 (vph)		
OPPOSING VOLUME-NOON (W/B Thru)	=	N/A (vph)		
OPPOSING VOLUME-PM (W/B Thru)	=	1236 (vph)		

OUTPUT

GUIDELINE A - MIN PEAK HOUR VOLS - AM WARRANT				
Product of opposing volume and LHT vol	=	15519	(vph)^2	
Warrant requirements	=	90000		
PERCENT FILLED	=	<u>17</u>	<u>%</u>	

N/A	(vph)^2
N/A	
<u>N/A</u>	<u>%</u>
-	= <u>N/A</u> = <u>N/A</u> = <u>N/A</u>

GUIDELINE A - MIN PEAK HOUR VOLS -PM WARRANT			
Product of opposing volume and LHT vol	=	35844	(vph)^2
Warrant requirements	=	90000	
PERCENT FILLED	=	<u>40</u>	<u>%</u>

LEFT TURN MIN PEAK HR VOL WARRANT GUIDELINE:	=	<u>40</u>	%
LEFT TURN VOL/SPEED WARRANT GUIDELINE A MET?:	=	NO	
LEFT TURN DELAY WARRANT GUIDELINE B MET ?:		NO	
LEFT TURN CRASH WARRANT GUIDELINE MET C?:	Ξ	NO	
LEFT TURN VOL WARRANT GUIDELINES MET UNDER A, B OR C?:	=	LT PHASE IS NOT WARRANTED	

<-Enter data in shaded cells

GUIDELINE B - A Delay calculated from Does the LHT vol ex **GUIDELINE B - N** Delay calculated from Does the LHT vol ex

GUIDELINE B - P Delay calculated from Does the LHT vol ex

GUIDELINE C - C NUMBER OF LHT DOES NUMBER H

GUIDELINE A - A Does the LHT vol ex

GUIDELINE A - 12 Does the LHT vol ex

GUIDELINE A - P Does the LHT vol ex

19 Traffic Volume for Southbound Left-Turn

AM DELAY WARRANT		
om Synchro for LHT movement =	0.19	hrs/peak hr
exceed warrant 'Guideline' B TEM =	NO	
NOON DELAY WARRANT		
om Synchro for LHT movement =	N/A	hrs/peak hr
exceed warrant 'Guideline' B TEM =	N/A	

PM DELAY WARRANT		
om Synchro for LHT movement =	0.3	6 hrs/peak hr
exceed warrant 'Guideline' B TEM =		

CRASH WARRANT		
T CRASHES IN 12 MONTHS? =	0	LT crashes
EXCEED 5 THEREFORE WARRANTS MET? =	NO	

AM SPEED WARRANT		
exceed warrant 'Guideline' A TEM =	NO	

exceed warrant 'Guideline A' TEM =	N/A	

PM SPEED WARRANT			
exceed warrant 'Guideline A' TEM	=	NO	

APPENDIX C

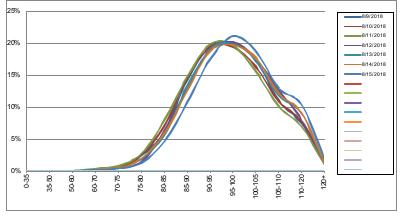
EXISTING VEHICLE SPEEDS AND TRAFFIC CLASSIFICATION DATA





Daily Speed B	Bin Pe	rcenta	iges												Daily	Spee	d Bin	Volu	mes										1	
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	Total	Avg.
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	00-105	05-110	10-120	120+	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	00-105	05-110	10-120	120+	Volume	peed
8/9/2018	0.0%	0.0%	0.0%	0.2%	0.6%	2.4%	6.2%	14.2%	19.5%	20.1%	17.3%	11.0%	7.4%	1.0%	1	3	2	39	95	391	991	2276	3117	3213	2768	1763	1193	163	16015	97
8/10/2018	0.0%	0.0%	0.0%	0.3%	0.6%	2.6%	7.0%	14.6%	19.4%	19.5%	16.3%	10.9%	7.7%	1.0%	1	0	4	46	97	441	1181	2438	3257	3260	2732	1828	1292	175	16752	97
8/11/2018	0.0%	0.0%	0.1%	0.4%	0.9%	2.7%	7.9%	14.7%	19.8%	19.7%	15.6%	10.1%	7.0%	1.1%	1	1	9	57	124	400	1153	2147	2891	2870	2270	1477	1018	156	14574	96
8/12/2018	0.0%	0.0%	0.0%	0.2%	0.5%	1.6%	6.2%	13.0%	18.9%	20.1%	17.6%	12.7%	8.0%	1.3%	1	0	2	28	75	237	937	1965	2870	3056	2670	1919	1216	193	15169	98
8/13/2018	0.0%	0.0%	0.0%	0.2%	0.5%	1.9%	5.7%	13.3%	19.0%	20.0%	17.3%	11.8%	9.1%	1.2%	1	0	1	26	78	301	874	2054	2939	3084	2664	1829	1400	190	15441	98
8/14/2018	0.0%	0.0%	0.0%	0.3%	0.5%	2.0%	5.9%	12.6%	18.8%	19.7%	17.7%	12.1%	9.1%	1.3%	4	1	2	43	76	311	908	1938	2899	3043	2733	1875	1407	197	15437	98
8/15/2018	0.0%	0.0%	0.0%	0.2%	0.4%	1.3%	4.7%	10.7%	17.5%	21.1%	18.7%	13.0%	10.4%	1.9%	3	0	6	32	70	208	739	1695	2764	3338	2963	2051	1641	300	15810	99
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Total	0.0%	0.0%	0.0%	0.2%	0.6%	2.1%	6.2%	13.3%	19.0%	20.0%	17.2%	11.7%	8.4%	1.3%	12	5	26	271	615	2289	6783	14513	20737	21864	18800	12742	9167	1374	109198	98

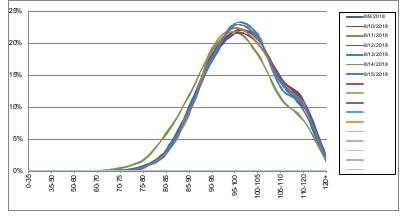
opeca bata ot		• •													
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	l
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	00-105	05-110	10-120	120+	Total
SurveyTotal	12	5	26	271	615	2289	6783	14513	20737	21864	18800	12742	9167	1374	1E+05
% Total	0.0%	0.0%	0.0%	0.2%	0.6%	2.1%	6.2%	13.3%	19.0%	20.0%	17.2%	11.7%	8.4%	1.3%	100%
Accumulated %	0.0%	0.0%	0.0%	0.3%	0.9%	2.9%	9.2%	22.4%	41.4%	61.5%	78.7%	90.3%	98.7%	100%	
Inverse Accum. %	#####	#####	#####	99.7%	99.1%	97.1%	90.8%	77.6%	58.6%	38.5%	21.3%	9.7%	1.3%	0.0%	1
Average Speed	98	KM∕H													





aily Speed E	in Pe	rcenta	ages												Daily	Spee	d Bin	Volu	mes											
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	Total	Αv
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	00-105	05-110	10-120	120+	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	05-110	10-120	120+	Volume	pee
8/9/2018	0.0%	0.0%	0.0%	0.0%	0.2%	0.7%	3.4%	10.1%	18.2%	21.8%	20.8%	13.8%	9.6%	1.4%	0	5	2	7	27	117	553	1654	3001	3595	3415	2274	1574	232	16456	5
8/10/2018	0.0%	0.0%	0.0%	0.0%	0.1%	0.5%	3.0%	9.3%	17.4%	22.0%	20.7%	14.5%	10.5%	1.9%	0	3	3	5	23	93	534	1629	3061	3863	3639	2556	1850	332	17591	10
8/11/2018	0.0%	0.0%	0.0%	0.1%	0.5%	1.8%	5.8%	11.8%	19.0%	21.8%	18.3%	11.4%	7.9%	1.5%	0	6	0	9	79	263	862	1748	2799	3225	2696	1684	1172	224	14767	' <u>(</u>
8/12/2018	0.0%	0.1%	0.0%		0.2%					21.5%				2.1%	0	11	1	9	25	124	415	1318	2423	3009	2787	2016	1538	287	13963	10
8/13/2018	0.0%	0.0%	0.0%	0.1%	0.1%	0.7%	3.1%	9.3%	17.6%	23.1%	21.3%	13.0%	10.2%	1.6%	0	3	2	11	12	101	462	1410	2662	3485	3217	1971	1534	237	15107	10
8/14/2018	0.0%	0.0%	0.0%	0.1%	0.1%	0.5%	2.9%	8.9%	19.1%	22.4%	19.9%	14.0%	10.3%	1.6%	0	2	1	14	19	83	452	1368	2940	3437	3061	2155	1582	243	15357	10
8/15/2018	0.0%	0.0%	0.0%	0.1%	0.2%	0.6%	2.7%	9.5%	17.5%	22.8%	20.8%	13.9%	10.2%	1.7%	0	7	3	8	29	97	432	1500	2761	3592	3272	2191	1601	261	15754	10
Total	0.0%	0.0%	0.0%	0.1%	0.2%	0.8%	3.4%	9.7%	18.0%	22.2%	20.3%	13.6%	10.0%	1.7%	0	37	12	63	214	878	3710	10627	19647	24206	22087	14847	10851	1816	108995	1

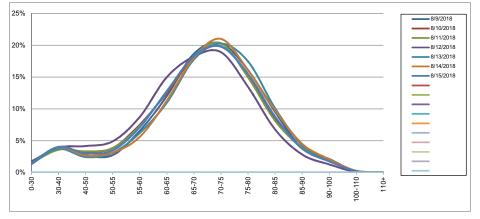
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Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	00-105	05-110	10-120	120+	Total
SurveyTotal	0	37	12	63	214	878	3710	10627	19647	24206	22087	14847	10851	1816	1E+05
% Total	0.0%	0.0%	0.0%	0.1%	0.2%	0.8%	3.4%	9.7%	18.0%	22.2%	20.3%	13.6%	10.0%	1.7%	100%
Accumulated %	0.0%	0.0%	0.0%	0.1%	0.3%	1.1%	4.5%	14.3%	32.3%	54.5%	74.8%	88.4%	98.3%	100%	
Inverse Accum. %	#####	#####	#####	99.9%	99.7%	98.9%	95.5%	85.7%	67.7%	45.5%	25.2%	11.6%	1.7%	0.0%	
Average Speed	100	KM∕H													





Daily Speed Bin	Percent	tages												1	Daily S	peed l	Bin Vo	lumes												
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	Total	Avg.
Bin Speeds	0-30	30-40	40-50	50-55	55-60	60-65	65-70	70-75	75-80	80-85	85-90	90-100	100-110	110+	0-30	30-40	40-50	50-55	55-60	60-65	65-70	70-75	75-80	80-85	85-90	90-100	100-110	110+	Volume	Speed
8/9/2018	1.3%	4.0%	2.7%	2.7%	6.5%	12.0%	18.7%	20.3%	16.1%	9.5%	3.8%	2.1%	0.2%	0.0%	190	571	391	394	943	1729	2688	2926	2318	1374	546	305	28	1	14404	69
8/10/2018	1.4%	3.7%	3.2%	3.8%	7.5%	12.5%	18.1%	19.8%	15.4%	8.4%	3.8%	1.9%	0.3%	0.1%	218	562	485	576	1130	1883	2723	2981	2311	1271	572	289	38	8	15047	68
8/11/2018	1.5%	3.6%	3.3%	3.8%	7.3%	12.7%	18.3%	20.3%	15.0%	8.1%	3.9%	1.9%	0.2%	0.0%	197	475	442	511	971	1691	2434	2700	1990	1082	523	249	30	4	13299	68
8/12/2018	1.8%	3.9%	4.2%	4.9%	8.7%	15.0%	18.2%	18.9%	13.5%	6.8%	2.8%	1.3%	0.1%	0.0%	250	554	589	693	1236	2123	2581	2681	1908	959	400	180	13	3	14170	66
8/13/2018	1.4%	3.7%	2.4%	3.1%	6.2%	11.0%	17.8%	20.3%	17.5%	10.1%	4.3%	2.0%	0.2%	0.0%	197	516	336	428	857	1515	2454	2800	2413	1396	592			3	13818	69
8/14/2018	1.5%	4.0%	2.7%	3.1%	5.6%	11.3%	18.0%	21.0%	16.1%	9.8%	4.5%	2.1%	0.2%	0.1%	208	542	364	428	764	1541	2466	2871	2202	1339	616	285	33	10	13669	69
8/15/2018	1.5%	4.0%	3.0%	3.5%	7.1%	12.8%	18.3%	19.8%	15.5%	8.8%	3.8%	1.7%	0.2%	0.0%	211	572	426	493	1006	1815	2591	2804	2197	1247	532	247	31	5	14177	68
															_						_		_							
	4 50/	0.00/	0.404	0.001	7.00/	10.5%	40.00/	00.00/	15.00/	0.001	0.001	1.00/	0.00/	0.00/		0700	0000	0.500	0007	10007	17007	10700	15000	0000	0704	1000	000	0.4	00504	
Total	1.5%	3.8%	3.1%	3.6%	7.0%	12.5%	18.2%	20.0%	15.6%	8.8%	3.8%	1.9%	0.2%	0.0%	1471	3792	3033	3523	6907	12297	17937	19763	15339	8668	3781	1833	206	34	98584	68

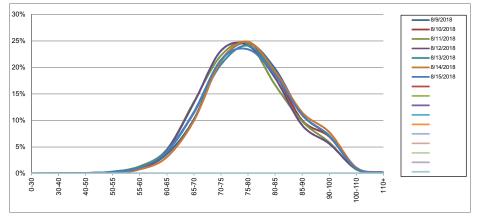
opoon sain out															
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Speeds	0-30	30-40	40-50	50-55	55-60	60-65	65-70	70-75	75-80	80-85	85-90	90-100	100-110	110+	Total
Survey Total	1471	3792	3033	3523	6907	12297	17937	19763	15339	8668	3781	1833	206	34	98584
% Total	1.5%	3.8%	3.1%	3.6%	7.0%	12.5%	18.2%	20.0%	15.6%	8.8%	3.8%	1.9%	0.2%	0.0%	100%
Accumulated %	1.5%	5.3%	8.4%	12.0%	19.0%	31.5%	49.7%	69.7%	85.3%	94.1%	97.9%	99.8%	100.0%	100%	
Inverse Accum. %	98.5%	94.7%	91.6%	88.0%	81.0%	68.5%	50.3%	30.3%	14.7%	5.9%	2.1%	0.2%	0.0%	0.0%	
Average Speed	68	KM/H													





Daily Speed Bin	Percen	tages													Daily S	Speed	Bin Vo	lumes												
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	Total	Avg.
Bin Speeds	0-30	30-40	40-50	50-55	55-60	60-65	65-70	70-75	75-80	80-85	85-90	90-100	100-110	110+	0-30	30-40	40-50	50-55	55-60	60-65	65-70	70-75	75-80	80-85	85-90	90-100 1	00-110	110+	Volume	Speed
8/9/2018	0.0%	0.0%	0.1%	0.2%	0.9%	3.7%	10.2%	21.2%	24.7%	19.8%	11.1%	7.0%	1.0%	0.1%	1	4	11	33	125	504	1412	2924	3404	2726	1530	972	134	17	13797	78
8/10/2018	0.0%	0.1%	0.1%	0.2%	1.0%	4.1%	11.5%	21.3%	24.8%	18.8%	10.0%	7.0%	1.0%	0.2%	0	10	9	29	154	617	1725	3204	3722	2822	1509	1044	148	27	15020	78
8/11/2018	0.0%	0.1%	0.1%	0.3%	1.4%	4.5%	13.5%	22.3%	24.5%	16.7%	9.8%	5.9%	0.7%	0.1%	5	8	13	42	171	569	1704	2804	3088	2106	1241	739	94	15	12599	77
8/12/2018	0.0%	0.0%	0.1%	0.3%	0.8%	4.4%	13.3%	23.2%	24.1%	18.0%	9.1%	5.6%	0.9%	0.1%	1	6	9	32	103	542	1629	2836	2954	2206	1115	688	106	16	12243	77
8/13/2018	0.0%	0.0%	0.1%	0.3%	1.3%	4.1%	11.4%	20.5%	24.1%	18.8%	11.0%	7.2%	0.9%	0.2%	1	2	10	44	159	516	1456	2612	3067	2396	1401	911	119	24	12718	78
8/14/2018	0.0%	0.0%	0.0%	0.2%	0.8%	3.1%	9.9%	21.2%	24.8%	19.3%	11.5%	7.8%	1.1%	0.2%	0	3	5	26	100	395	1275	2715	3183	2469	1477	1001	144	24	12817	79
8/15/2018	0.0%	0.0%	0.0%	0.2%	1.1%	4.3%	11.6%	21.4%	23.4%	18.6%	11.1%	7.1%	1.0%	0.2%	1	4	6	33	146	565	1543	2850	3114	2473	1471	938	130	20	13294	78
																														i
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Total	0.0%	0.0%	0.1%	0.3%	1.0%	4.0%	11.6%	21.6%	24.4%	18.6%	10.5%	6.8%	0.9%	0.2%	9	37	63	239	958	3708	10744	19945	22532	17198	9744	6293	875	143	92488	78

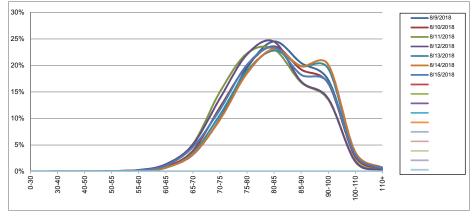
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	i i
Bin Speeds	0-30	30-40	40-50	50-55	55-60	60-65	65-70	70-75	75-80	80-85	85-90	90-100	100-110	110+	Total
Survey Total	9	37	63	239	958	3708	10744	19945	22532	17198	9744	6293	875	143	92488
% Total	0.0%	0.0%	0.1%	0.3%	1.0%	4.0%	11.6%	21.6%	24.4%	18.6%	10.5%	6.8%	0.9%	0.2%	100%
Accumulated %	0.0%	0.0%	0.1%	0.4%	1.4%	5.4%	17.0%	38.6%	63.0%	81.6%	92.1%	98.9%	99.8%	100%	
Inverse Accum. %	100.0%	100.0%	99.9%	99.6%	98.6%	94.6%	83.0%	61.4%	37.0%	18.4%	7.9%	1.1%	0.2%	0.0%	
Average Speed	78	KM/H													





Daily Speed Bin	Percen	tages													Daily S	Speed I	Bin Vo	lumes												
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	Total	Avg.
Bin Speeds	0-30	30-40	40-50	50-55	55-60	60-65	65-70	70-75	75-80	80-85	85-90	90-100	100-110	110+	0-30	30-40	40-50	50-55	55-60	60-65	65-70	70-75	75-80	80-85	85-90	90-100	100-110	110+	Volume	Speed
8/9/2018	0.0%	0.0%	0.0%	0.0%	0.1%	0.8%	3.2%	10.0%	19.6%	24.5%	20.5%	17.3%	3.2%	0.7%	1	3	4	4	22	137	515	1616	3173	3980	3322	2804	524	121	16226	84
8/10/2018	0.0%	0.0%	0.0%	0.1%	0.3%	0.9%	3.9%	12.1%	19.9%	23.6%	19.2%	16.6%	2.8%	0.4%	0	4	4	14	46	151	647	2004	3300	3906	3184	2749	461	73	16543	83
8/11/2018	0.0%	0.0%	0.0%	0.1%	0.3%	1.4%	5.2%	15.0%	22.3%	23.1%	16.8%	13.5%	2.1%	0.3%	0	3	0	13	39	200	756	2200	3261	3376	2460	1982	306	40	14636	82
8/12/2018	0.0%	0.0%	0.0%	0.0%	0.3%	1.4%	5.1%	13.8%	22.1%	24.5%	17.0%	13.8%		0.2%	1	4	2	7	42	208	782	2123	3400	3770	2613	2117	286	38	15393	82
8/13/2018	0.0%	0.0%	0.0%	0.0%	0.1%	0.7%	3.5%	10.7%	19.0%	22.8%	19.9%	19.4%	3.3%	0.6%	1	3	1	2	15	107	535	1632	2889	3475	3025	2959	502	86	15232	84
8/14/2018	0.0%	0.0%	0.0%	0.0%		0.7%	3.3%	9.8%	18.5%	23.1%	19.9%	20.2%	3.7%	0.6%	0	1	1	4	27	111	496	1483	2809	3506	3014	3059	555	98	15164	84
8/15/2018	0.0%	0.0%	0.0%	0.1%	0.2%	1.1%	4.6%	11.7%	20.1%	23.4%	18.2%	16.7%	3.1%	0.6%	0	5	6	15	34	176	730	1853	3193	3710	2893	2654	499	89	15857	83
																														
	0.00/	0.00/	0.00/	0.404	0.00/	1.00/	1 101	11.00/	00.00/	00.00/	40.00/	10.00/	0.00/	0.50/			10	50	005	1000		10011	00005	05700	00544	10001	0.400	5.45	100051	
Total	0.0%	0.0%	0.0%	0.1%	0.2%	1.0%	4.1%	11.8%	20.2%	23.6%	18.8%	16.8%	2.9%	0.5%	3	23	18	59	225	1090	4461	12911	22025	25723	20511	18324	3133	545	109051	83

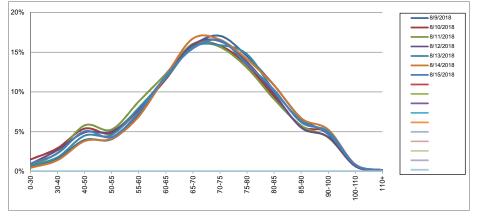
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Speeds	0-30	30-40	40-50	50-55	55-60	60-65	65-70	70-75	75-80	80-85	85-90	90-100	100-110	110+	Total
Survey Total	3	23	18	59	225	1090	4461	12911	22025	25723	20511	18324	3133	545	109051
% Total	0.0%	0.0%	0.0%	0.1%	0.2%	1.0%	4.1%	11.8%	20.2%	23.6%	18.8%	16.8%	2.9%	0.5%	100%
Accumulated %	0.0%	0.0%	0.0%	0.1%	0.3%	1.3%	5.4%	17.2%	37.4%	61.0%	79.8%	96.6%	99.5%	100%	
Inverse Accum. %	100.0%	100.0%	100.0%	99.9%	99.7%	98.7%	94.6%	82.8%	62.6%	39.0%	20.2%	3.4%	0.5%	0.0%	
Average Speed	83	KM/H													





Daily Speed Bin I	Percen	tages													Daily S	Speed E	Bin Vo	lumes												
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	Total	Avg
Bin Speeds	0-30	30-40	40-50	50-55	55-60	60-65	65-70	70-75	75-80	80-85	85-90	90-100	100-110	110+	0-30	30-40	40-50	50-55	55-60	60-65	65-70	70-75	75-80	80-85	85-90	90-100	100-110	110+	Volume	Speer
8/9/2018	0.7%	1.7%	4.0%	4.1%	7.1%	12.0%	15.8%	17.1%	14.5%	10.8%	6.7%	4.7%	0.8%	0.1%	104	255	610	627	1097	1856	2438	2632	2235	1673	1033	722	125	22	15429	7
8/10/2018	1.5%	2.9%	5.4%	4.8%	7.8%	11.6%	15.9%	15.8%	13.4%	9.4%	5.7%	4.9%	0.7%	0.1%	253	494	910	810	1311	1954	2676	2668	2255	1591	960	826	124	19	16851	6
8/11/2018	0.7%	2.3%	5.8%	5.3%	8.8%	12.3%	16.0%	15.7%	13.0%	9.1%	5.8%	4.3%	0.8%	0.1%	101	332	822	748	1247	1741	2273	2221	1848	1290	820	605	111	14	14173	6
8/12/2018	0.9%	2.7%	4.9%	5.0%	7.9%	11.9%	16.0%	16.4%	13.8%	9.8%	5.5%	4.3%	0.7%	0.2%	126	360	656	672	1053	1594	2131	2188	1837	1304	738	579	88	25	13351	6
8/13/2018	0.7%	1.8%	4.5%	4.6%	8.0%	11.7%	15.6%	15.9%	14.7%	10.1%	6.4%	4.8%	0.8%	0.2%	106	256	643	658	1144	1676	2234	2271	2106	1450	919			22	14291	70
8/14/2018	0.5%	1.4%	3.8%	4.1%	6.9%	12.0%	16.7%	16.6%	14.1%	10.8%	6.7%	5.3%	0.9%	0.1%	66	205	552	597	999	1736	2414	2394	2036	1563	967	761	129	20	14439	71
8/15/2018	1.0%	2.3%	5.1%	4.3%	7.4%	12.0%	15.5%	16.5%	13.5%	10.3%	6.2%	5.0%	0.9%	0.1%	150	346	768	658	1130	1822	2349	2502	2048	1558	940	762	133	16	15182	70
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Total	0.9%	2.2%	4.8%	4.6%	7 7%	11.9%	15.9%	16.3%	13.9%	10.1%	6.1%	4.8%	0.8%	0.1%	906	2248	4961	4770	7981	12379	16515	16876	14365	10429	6377	4941	830	138	103716	7

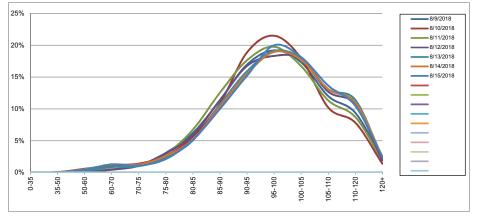
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Speeds	0-30	30-40	40-50	50-55	55-60	60-65	65-70	70-75	75-80	80-85	85-90	90-100	100-110	110+	Total
Survey Total	906	2248	4961	4770	7981	12379	16515	16876	14365	10429	6377	4941	830	138	103716
% Total	0.9%	2.2%	4.8%	4.6%	7.7%	11.9%	15.9%	16.3%	13.9%	10.1%	6.1%	4.8%	0.8%	0.1%	100%
Accumulated %	0.9%	3.0%	7.8%	12.4%	20.1%	32.1%	48.0%	64.2%	78.1%	88.2%	94.3%	99.1%	99.9%	100%	
Inverse Accum. %	99.1%	97.0%	92.2%	87.6%	79.9%	67.9%	52.0%	35.8%	21.9%	11.8%	5.7%	0.9%	0.1%	0.0%	
Average Speed	70	KM/H													





Daily Speed Bin	Percent	tages													Daily S	Speed E	Bin Vo	lumes												
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	Total	Avg.
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	Volume	Speed
8/9/2018	0.0%	0.0%	0.3%	1.0%	1.2%	3.1%	6.2%	11.3%	17.0%	19.2%	17.3%	12.0%	9.4%	2.0%	0	5	41	130	155	405	815	1488	2230	2524	2278	1579	1234	265	13149	98
8/10/2018	0.0%	0.1%	0.6%	1.1%	1.4%	2.6%	5.6%	10.8%	18.9%	21.5%	17.9%	10.2%	7.8%	1.4%	1	8	80	155	193	365	783	1508	2625	2989	2495	1420	1089	191	13902	97
8/11/2018	0.0%	0.0%	0.2%	0.5%	1.1%	2.9%	6.7%	12.5%	17.6%	19.8%	16.7%	11.3%	8.7%	1.9%	0	1	19	63	133	353	822	1532	2155	2414	2042	1383	1064	232	12213	97
8/12/2018	0.0%	0.0%	0.1%	0.4%	1.1%	3.0%	5.9%	11.4%	16.7%	18.3%	17.8%	12.7%	10.7%	1.9%	1	2	8	57	135	385	757	1462	2149	2352	2286	1629	1367	240	12830	98
8/13/2018	0.0%	0.0%	0.3%	0.8%	1.0%	2.3%	5.0%	10.7%	15.9%	19.1%	18.0%	13.1%	11.4%	2.3%	2	3	33	108	127	294	642	1355	2026	2432	2284	1667	1451	297	12721	99
8/14/2018	0.0%	0.0%	0.5%	1.2%	1.3%	2.6%	5.2%	10.4%	15.5%	18.9%	17.7%	13.1%	11.0%	2.5%	0	5	64	155	160	329	657	1316	1965	2399	2242	1654	1396	318	12660	98
8/15/2018	0.0%	0.0%	0.4%	1.3%	1.1%	2.1%	5.1%	10.0%	15.2%	20.0%	18.1%	13.7%	10.5%	2.5%	0	4	58	168	140	277	672	1310	1992	2619	2375	1788	1371	324	13098	99
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Total	0.0%	0.0%	0.3%	0.9%	1.2%	2.7%	5.7%	11.0%	16.7%	19.6%	17.7%	12.3%	9.9%	2.1%	4	28	303	836	1043	2408	5148	9971	15142	17729	16002	11120	8972	1867	90573	98

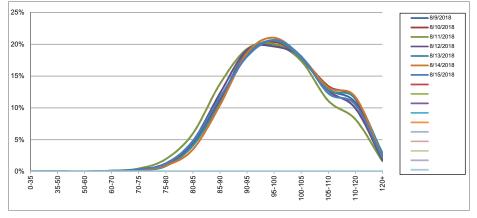
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	Total
Survey Total	4	28	303	836	1043	2408	5148	9971	15142	17729	16002	11120	8972	1867	90573
% Total	0.0%	0.0%	0.3%	0.9%	1.2%	2.7%	5.7%	11.0%	16.7%	19.6%	17.7%	12.3%	9.9%	2.1%	100%
Accumulated %	0.0%	0.0%	0.4%	1.3%	2.4%	5.1%	10.8%	21.8%	38.5%	58.1%	75.8%	88.0%	97.9%	100%	
Inverse Accum. %	100.0%	100.0%	99.6%	98.7%	97.6%	94.9%	89.2%	78.2%	61.5%	41.9%	24.2%	12.0%	2.1%	0.0%	
Average Speed	98	KM/H													





Daily Speed Bin I	Percent	ages													Daily S	peed l	Bin Vo	lumes												
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	Total	Avg.
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	Volume	Speed
8/9/2018	0.0%	0.0%	0.0%	0.1%	0.3%	1.3%	4.4%	11.3%	18.7%	20.0%	17.6%	12.9%	10.8%	2.5%	3	4	1	14	33	165	579	1473	2442	2604	2296	1688	1411	324	13037	99
8/10/2018	0.0%	0.0%	0.0%	0.1%	0.3%	1.1%	4.1%	11.0%	18.2%	20.4%	17.8%	13.4%	11.4%	2.2%	1	4	0	10	38	163	583	1564	2599	2905	2543	1917	1626	320	14273	99
8/11/2018	0.0%	0.0%	0.0%	0.1%	0.5%	1.9%	6.0%	13.8%	19.3%	20.1%	17.4%	11.1%	8.2%	1.6%	1	4	1	14	56	236	723	1667	2332	2428	2106	1347	994	196	12105	98
8/12/2018	0.0%	0.0%	0.0%	0.1%	0.3%	1.3%	4.8%	12.3%	19.1%	19.6%	18.0%	12.7%	9.9%	1.8%	1	2	0	6	38	143	542	1388	2159	2220	2032	1439	1124	206	11300	99
8/13/2018	0.0%	0.0%	0.0%	0.1%	0.2%	0.8%	4.1%	10.8%	18.3%	20.6%	17.8%	12.9%	11.5%	2.9%	0	2	1	10		101	492	1292	2201	2472		1542	1378	352	12000	100
8/14/2018	0.0%	0.0%	0.0%	0.1%	0.2%	0.9%	3.5%	10.4%	18.6%	21.0%	18.0%	13.2%	11.7%	2.2%	0	1	0	12	25	113	419	1259	2251	2540	2179	1596	1417	268	12080	100
8/15/2018	0.0%	0.0%	0.0%	0.1%	0.3%	1.2%	4.8%	11.7%	18.1%	20.7%	18.1%	12.3%	10.5%	2.2%	1	5	0	13	44	152	606	1477	2286	2618	2291	1556	1332	276	12657	99
Total	0.0%	0.0%	0.0%	0.1%	0.3%	1.2%	4.5%	11.6%	18.6%	20.3%	17.8%	12.7%	10.6%	2.2%	7	22	3	79	258	1073	3944	10120	16270	17787	15580	11085	9282	1942	87452	99

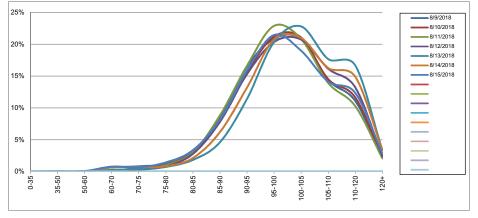
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	Total
Survey Total	7	22	3	79	258	1073	3944	10120	16270	17787	15580	11085	9282	1942	87452
% Total	0.0%	0.0%	0.0%	0.1%	0.3%	1.2%	4.5%	11.6%	18.6%	20.3%	17.8%	12.7%	10.6%	2.2%	100%
Accumulated %	0.0%	0.0%	0.0%	0.1%	0.4%	1.6%	6.2%	17.7%	36.3%	56.7%	74.5%	87.2%	97.8%	100%	
Inverse Accum. %	100.0%	100.0%	100.0%	99.9%	99.6%	98.4%	93.8%	82.3%	63.7%	43.3%	25.5%	12.8%	2.2%	0.0%	
Average Speed	99	KM/H													





Daily Speed Bin	Percen	tages													Daily	Speed	Bin Vo	lumes												
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	Total	Avg.
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	Volume	Speed
8/9/2018	0.0%	0.1%	0.0%	0.6%	0.8%	1.2%	3.0%	8.4%	16.5%	20.9%	20.9%	14.4%	11.1%	2.2%	1	7	6	88	111	161	411	1138	2241	2836	2835	1954	1507	301	13597	100
8/10/2018	0.0%	0.0%	0.0%	0.7%	0.6%	1.1%	3.2%	8.2%	15.6%	21.2%	21.1%	14.5%	11.6%	2.2%	0	0	4	102	90	154	455	1175	2247	3052	3030	2089	1673	316	14387	100
8/11/2018	0.0%	0.0%	0.0%	0.2%	0.3%	1.0%	3.0%	8.8%	16.6%	22.9%	20.8%	13.9%	10.3%	2.1%	0	1	3	29	43	130	383	1124	2122	2921	2656	1776	1309	263	12760	100
8/12/2018	0.0%	0.0%	0.1%	0.1%	0.3%	0.8%	2.8%	7.8%	15.3%	20.4%	20.7%	16.2%		2.4%	0	0	7	17	40	108	372	1049	2062	2740	2791	2174	1777	324	13461	101
8/13/2018	0.0%	0.0%	0.0%	0.2%	0.4%	0.7%	1.8%	4.6%	11.4%	20.4%	22.8%	17.7%		3.3%	0	0	1	33	49	96	241	609	1515	2699		2343	2209	443	13261	103
8/14/2018	0.0%	0.0%	0.0%			0.8%	2.1%				21.0%			3.4%	1	1	2	83	89	108	274	802	1709	2688		2107	1927	443	12950	102
8/15/2018	0.0%	0.0%	0.1%	0.8%	0.7%	1.4%	3.3%	8.1%	15.9%	21.5%	19.0%	14.1%	12.3%	2.8%	0	0	8	102	89	192	451	1097	2139	2891	2557	1902	1662	384	13474	100
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	0.00/	0.00/	0.00/	0.50(0.50/	1.00/	0.001	7 404	11.00/	04.404	00.00/	45.00/	10.00/	0.00/		-		15.4	511	0.40	0507	0004	4 4 9 9 5	10007	40000	1 10 15	10001	0.17.1	00000	101
Total	0.0%	0.0%	0.0%	0.5%	0.5%	1.0%	2.8%	7.4%	14.9%	21.1%	20.9%	15.3%	12.8%	2.6%	2	9	31	454	511	949	2587	6994	14035	19827	19608	14345	12064	2474	93890	101

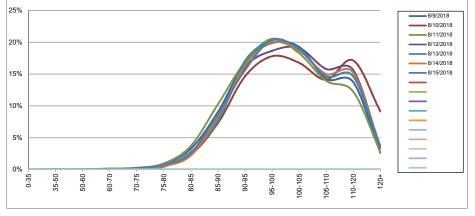
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	Total
Survey Total	2	9	31	454	511	949	2587	6994	14035	19827	19608	14345	12064	2474	93890
% Total	0.0%	0.0%	0.0%	0.5%	0.5%	1.0%	2.8%	7.4%	14.9%	21.1%	20.9%	15.3%	12.8%	2.6%	100%
Accumulated %	0.0%	0.0%	0.0%	0.5%	1.1%	2.1%	4.8%	12.3%	27.2%	48.4%	69.2%	84.5%	97.4%	100%	
Inverse Accum. %	100.0%	100.0%	100.0%	99.5%	98.9%	97.9%	95.2%	87.7%	72.8%	51.6%	30.8%	15.5%	2.6%	0.0%	
Average Speed	101	KM/H													





Daily Speed Bin	Percen	tages													Daily S	peed l	Bin Vo	lumes												
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	Total	Avg.
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	Volume	Speed
8/9/2018	0.0%	0.0%	0.0%	0.1%	0.2%	0.9%	3.3%	9.0%	16.8%	19.9%	19.3%	14.2%	13.7%	2.6%	2	2	3	11	31	117	439	1218	2261	2681	2598	1919	1852	353	13487	101
8/10/2018	0.0%	0.0%	0.0%	0.0%	0.2%	0.6%	2.3%	7.3%	14.6%	17.8%	16.8%	14.1%	17.1%	9.1%	0	3	0	7	24	87	351	1097	2205	2681	2524	2123	2579	1375	15056	104
8/11/2018	0.0%	0.0%	0.0%	0.1%	0.3%	1.0%	3.7%	10.3%	16.9%	20.5%	18.3%	13.9%	12.2%	2.8%	0	1	6	18	33	119	462	1280	2107	2565	2282	1739	1525	347	12484	100
8/12/2018	0.0%	0.0%	0.0%	0.0%	0.2%	0.6%	2.6%	8.0%	16.1%	18.7%	19.1%	15.8%	15.6%	3.4%	0	0	0	4	21	75	297	927	1865	2167	2219	1835	1813	396	11619	102
8/13/2018	0.0%	0.0%	0.0%	0.0%	0.1%	0.4%	2.7%	7.6%	17.1%	20.1%	19.0%	14.7%	14.6%	3.6%	0	3	1	2	15	54	340	937	2114	2485	2352	1816	1810	451	12380	101
8/14/2018	0.0%	0.0%	0.0%	0.0%	0.1%	0.5%	2.2%	8.2%	16.3%	20.0%	18.7%	15.0%	15.2%	3.7%	0	2	0	5	13	66	277	1023	2037	2494	2330	1872	1896	462	12477	102
8/15/2018	0.0%	0.0%	0.0%	0.1%	0.1%	0.8%	2.7%	8.7%	15.6%	20.4%	18.9%	14.6%	14.6%	3.6%	2	4	1	7	15	99	347	1137	2046	2669	2472	1914	1915	466	13094	101
																														1
Total	0.0%	0.0%	0.0%	0.1%	0.2%	0.7%	2.8%	8.4%	16.2%	19.6%	18.5%	14.6%	14.8%	4.2%	4	15	11	54	152	617	2513	7619	14635	17742	16777	13218	13390	3850	90597	102

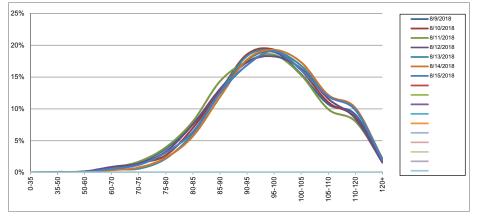
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	Total
Survey Total	4	15	11	54	152	617	2513	7619	14635	17742	16777	13218	13390	3850	90597
% Total	0.0%	0.0%	0.0%	0.1%	0.2%	0.7%	2.8%	8.4%	16.2%	19.6%	18.5%	14.6%	14.8%	4.2%	100%
Accumulated %	0.0%	0.0%	0.0%	0.1%	0.3%	0.9%	3.7%	12.1%	28.3%	47.9%	66.4%	81.0%	95.8%	100%	
Inverse Accum. %	100.0%	100.0%	100.0%	99.9%	99.7%	99.1%	96.3%	87.9%	71.7%	52.1%	33.6%	19.0%	4.2%	0.0%	
Average Speed	102	KM/H													





Daily Speed Bin	Percen	tages												I	Daily S	peed l	Bin Vo	lumes												
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	Total	Avg.
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	Volume	Speed
8/9/2018	0.0%	0.1%	0.2%	0.9%	1.6%	3.7%	7.6%	12.4%	17.7%	19.0%	15.4%	10.7%	9.1%	1.7%	1	12	20	112	204	486	985	1621	2301	2469	2012	1398	1186	222	13029	97
8/10/2018	0.0%	0.0%	0.1%	0.7%	1.5%	2.7%	7.0%	12.8%	18.6%	19.3%	16.1%	11.0%	8.7%	1.6%	0	1	15	94	202	368	954	1748	2534	2633	2194	1507	1182	213	13645	97
8/11/2018	0.0%	0.1%	0.2%	0.8%	1.7%	4.0%	8.0%	14.3%	17.4%	18.5%	15.3%	9.9%	8.0%	1.7%	0	7	22	96	204	476	963	1719	2093	2221	1836	1193	965	201	11996	96
8/12/2018	0.0%	0.0%	0.1%	0.9%	1.4%	3.3%	7.6%	13.1%	17.3%	18.3%	16.3%	11.6%	8.5%	1.6%	0	5	18	109	179	414	950	1647	2178	2294	2050	1457	1067	200	12568	97
8/13/2018	0.0%	0.0%	0.0%	0.5%	0.6%	2.2%	6.1%	12.5%	18.3%	19.1%	16.8%	12.1%	9.8%	2.0%	0	0	3	62	75	275	770	1562	2293	2396	2103	1511	1231	247	12528	98
8/14/2018	0.0%	0.0%	0.1%	0.4%	0.8%	2.3%	5.7%	11.9%	17.7%	19.3%	17.3%	12.2%	10.2%	2.2%	0	0	7	44	95	285	697	1451	2171	2357	2120	1491	1243	270	12231	98
8/15/2018	0.0%	0.0%	0.1%	0.6%	1.2%	3.1%	6.4%	12.5%	16.9%	19.1%	16.1%	12.0%	9.9%	2.1%	0	6	13	75	152	397	817	1594	2150	2433	2051	1525	1258	270	12741	98
Total	0.0%	0.0%	0.1%	0.7%	1.3%	3.0%	6.9%	12.8%	17.7%	18.9%	16.2%	11.4%	9.2%	1.8%	1	31	98	592	1111	2701	6136	11342	15720	16803	14366	10082	8132	1623	88738	97

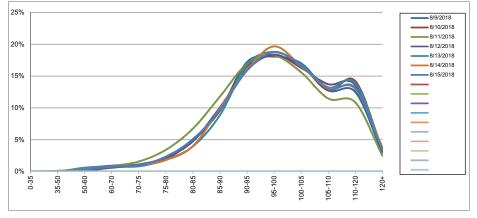
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	Total
Survey Total	1	31	98	592	1111	2701	6136	11342	15720	16803	14366	10082	8132	1623	88738
% Total	0.0%	0.0%	0.1%	0.7%	1.3%	3.0%	6.9%	12.8%	17.7%	18.9%	16.2%	11.4%	9.2%	1.8%	100%
Accumulated %	0.0%	0.0%	0.1%	0.8%	2.1%	5.1%	12.0%	24.8%	42.5%	61.5%	77.6%	89.0%	98.2%	100%	
Inverse Accum. %	100.0%	100.0%	99.9%	99.2%	97.9%	94.9%	88.0%	75.2%	57.5%	38.5%	22.4%	11.0%	1.8%	0.0%	
Average Speed	97	KM/H													





Daily Speed Bin	Percent	tages													Daily S	peed l	Bin Vo	lumes												
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	Total	Avg.
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	Volume	Speed
8/9/2018	0.0%	0.0%	0.5%	0.9%	1.1%	2.2%	5.1%	9.6%	17.2%	18.1%	17.0%	12.8%	12.5%	3.0%	2	3	66	126	152	305	696	1316	2350	2467	2322	1743	1706	406	13660	99
8/10/2018	0.0%	0.0%	0.3%	0.8%	0.9%	2.0%	4.8%	9.6%	16.5%	18.2%	16.7%	12.7%	14.2%	3.4%	1	2	37	114	131	294	710	1403	2422	2668	2449	1872	2087	500	14690	100
8/11/2018	0.0%	0.0%	0.1%	0.8%	1.6%	3.4%	6.7%	11.8%	16.9%	18.3%	15.6%	11.5%	10.9%	2.5%	1	0	15	104	195	423	838	1467	2106	2274	1941	1429	1352	310	12455	98
8/12/2018	0.0%	0.0%	0.1%	0.6%	1.0%	2.2%	4.7%	10.1%	16.0%	18.4%	16.3%	13.7%	13.6%	3.2%	0	4	16	69	114	258	550	1168	1851	2128	1887	1589	1574	374	11582	100
8/13/2018	0.0%	0.0%	0.6%	0.8%	0.8%	1.9%	4.0%	8.9%	16.8%	18.8%	16.8%	13.2%	13.9%	3.6%	0	2	74	102	101	232	492	1103	2092	2339	2090	1647	1732	444	12450	100
8/14/2018	0.0%	0.1%	0.4%	0.8%	1.0%	1.8%	4.0%	9.9%	16.0%	19.7%	16.6%	13.1%	13.0%	3.8%	0	12	45	97	121	225	499	1242	2000	2464	2075	1641	1625	479	12525	100
8/15/2018	0.0%	0.0%	0.3%	0.8%	1.0%	2.3%	5.0%	9.7%	15.9%	18.8%	16.7%	13.0%	13.1%	3.4%	1	0	42	108	127	308	664	1275	2087	2469	2197	1704	1727	445	13154	99
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Total	0.0%	0.0%	0.3%	0.8%	1.0%	2.3%	4.9%	9.9%	16.5%	18.6%	16.5%	12.8%	13.0%	3.3%	5	23	295	720	941	2045	4449	8974	14908	16809	14961	11625	11803	2958	90516	99

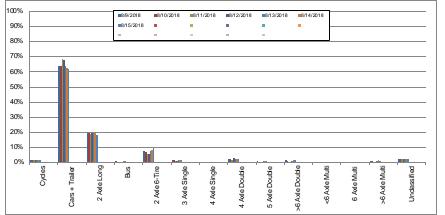
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Speeds	0-35	35-50	50-60	60-70	70-75	75-80	80-85	85-90	90-95	95-100	100-105	105-110	110-120	120+	Total
Survey Total	5	23	295	720	941	2045	4449	8974	14908	16809	14961	11625	11803	2958	90516
% Total	0.0%	0.0%	0.3%	0.8%	1.0%	2.3%	4.9%	9.9%	16.5%	18.6%	16.5%	12.8%	13.0%	3.3%	100%
Accumulated %	0.0%	0.0%	0.4%	1.2%	2.2%	4.5%	9.4%	19.3%	35.8%	54.3%	70.8%	83.7%	96.7%	100%	
Inverse Accum. %	100.0%	100.0%	99.6%	98.8%	97.8%	95.5%	90.6%	80.7%	64.2%	45.7%	29.2%	16.3%	3.3%	0.0%	
Average Speed	99	KM/H													





ly Class	Bin F	Perc	en	tage	5											Daily	Clas	s Bin	Volu	mes										
Bin #		1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
	Cycle	э	+	Axle		Axle	Axle	Axle	Doubl	Doubl	Axle	Axle	Axle	Axle	ssifie	Cycle	+	Axle		Axle	Axle	Axle	Doubl	Doubl	Axle	Axle	6 Axle	Axle s	sifie	
Bin Class		s Tr	aile	Long	Bus	6-Tire	Singl	Singl	е	е	Doubl	Multi	Multi	Multi	d	s	Traile	Long	Bus	6-Tire	Singl	Singl	е	е	Doubl	Multi	Multi	Multi	d	ν
8/9/2018	1.2%	63	4%	19.0%	0.8%	7.4%	1.0%	0.5%	2.0%	0.8%	1.1%	0.1%	0.0%	0.9%	1.9%	189	10147	3035	121	1187	168	73	327	121	180	14	4	152	297	1
/10/2018	1.3%	63	5%	19.3%	0.6%	6.8%	1.1%	0.5%	2.1%	0.7%	0.9%	0.1%	0.1%	0.7%	2.2%	219	10636	3239	96	1136	190	88	358	110	158	24	14	121	363	1
3/11/2018	1.2%	68	.3%	18.6%	0.5%	5.6%	0.9%	0.4%	1.6%	0.3%	0.4%	0.1%	0.0%	0.3%	1.8%	180	9949	2714	67	816	131	59	240	40	56	13	3	37	269	1
/12/2018	1.5%	667	3%	18.9%	0.2%	5.3%	0.8%	0.5%	2.4%	0.4%	0.2%	0.1%	0.1%	0.2%	2.1%	229	10207	2867	30	801	117	83	363	56	34	22	8	32	320	F
3/13/2018	1.3%	63	.6%	19.2%	0.6%	7.1%	1.2%	0.4%	1.9%	0.7%	1.0%	0.1%	0.1%	1.0%	2.1%	203	9817	2958	91	1091	178	64	291	107	156	9	9	150	317	Ē
/14/2018	1.3%	62	.0%	19.0%	0.8%	8.3%	1.2%	0.4%	2.1%	0.7%	1.1%	0.1%	0.1%	1.1%	1.9%	198	9574	2938	118	1278	179	58	329	110	175	9		163	292	ľ
3/15/2018	1.3%	661	8%	18.0%	0.8%	9.2%	1.4%	0.4%	2.0%	0.7%	1.0%	0.1%	0.0%	1.0%	2.2%	202	9777	2841	132	1452	224	64	319	110	163	9	4	159	354	1
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Total	1.3%	6 64	2%	18.9%	0.6%	7.1%	1.1%	0.4%	2.0%	0.6%	0.8%	0.1%	0.1%	0.7%	2.0%	1420	70107	20592	655	7761	1187	489	2227	654	922	100	58	814	2212	Г

Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
	Cycle	+	Axle		Axle	Axle	Axle	Doubl	Doubl	Axle	Axle	Axle	Axle	ssifie	Volum
Bin Class	s	Traile	Long	Bus	6-Tire	Singl	Singl	e	е	Doubl	Multi	Multi	Multi	d	е
SurveyTotal	1420	70107	20592	655	7761	1187	489	2227	654	922	100	58	814	2212	109198
% Total	1.3%	64.2%	18.9%	0.6%	7.1%	1.1%	0.4%	2.0%	0.6%	0.8%	0.1%	0.1%	0.7%	2.0%	100%
Accumulated %	1.3%	65.5%	84.4%	85.0%	92.1%	93.2%	93.6%	95.6%	96.2%	97.1%	97.2%	97.2%	98.0%	100.0%	
Inv. Accum. %	98.7%	34.5%	15.6%	15.0%	7.9%	6.8%	6.4%	4.4%	3.8%	2.9%	2.8%	2.8%	2.0%	0.0%	



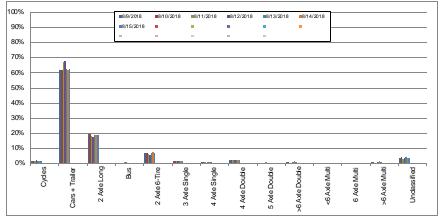
Classification Descriptions

	Sincaron Descriptions
Bin #	Bin Description
1	Motorcycles
2	Passenger cars, including those with recreational trailers
3	2-axle pick-ups, vans, and RVs, including those with recreational trailers
4	Buses
5	2-axle, 6-tire single units
6	3-axle single units
7	4-axle single units
8	3-axle or 4-axle double unit
9	5-axle double unit
10	6-, 7-, or 8-axle double unit
11	5-axle multiple unit
12	6-axle multiple unit
13	7-or-more-axle multiple unit
14	Vehicles that could not be classified

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ily Class I	Bin I	Perc	cen	tage	s												Daily	Clas	s Bin	Volu	mes										
Bin #		1	2		3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
	Cycl	e	+	Axle	9	Î	Axle	Axle	Axle	Doubl	Doubl	Axle	Axle	Axle	Axle	ssifie	Cycle	+	Axle		Axle	Axle	Axle	Doubl	Doubl	Axle	Axle	6 Axle	Axle s	sifie	
Bin Class		s Tr	aile	Long) В	us	6-Tire	Singl	Singl	е	е	Doubl	Multi	Multi	Multi	d	s	Traile	Long	Bus	6-Tire	Singl	Singl	е	е	Doubl	Multi	Multi	Multi	d	ν
8/9/2018	1.5%	661	.9%	19.1%	6 0.6	8%	7.0%	1.2%	0.8%	2.0%	0.6%	1.0%	0.0%	0.1%	0.8%	3.6%	248	10187	3144	106	1147	196	126	321	94	159	6	13	124	585	1
3/10/2018	1.5%	661	.8%	19.3%	6 0.5	%	6.6%	1.2%	0.8%	2.2%	0.5%	0.9%	0.1%	0.1%	0.8%	3.7%	269	10867	3395	96	1165	204	145	394	88	157	9	18	132	652	1
8/11/2018	1.3%	66	.6%	18.0%	6 0.4	%	6.0%	1.3%	0.7%	1.8%	0.4%	0.4%	0.0%	0.0%	0.3%	2.8%	185	9837	2653	53	892	189	103	260	60	65	5	7	44	414	1
/12/2018	1.79	667	.4%	17.49	6 0.2	%	5.6%	1.1%	0.7%	1.9%	0.4%	0.2%	0.0%	0.1%	0.2%	3.3%	240	9412	2429	29	775	147	92	259	53	34	4	13	21	455	1
3/13/2018	1.4%	6 62	.2%	18.7%	6 0.5	5%	6.7%	1.4%	0.7%	1.9%	0.6%	1.0%	0.0%	0.1%	0.9%	3.9%	219	9401	2831	71	1014	204	102	289	89	150	7	8	136	586	Ē
/14/2018	1.5%	661	.4%	18.6%	6 0.7	%	7.1%	1.2%	0.9%	2.0%	0.8%	1.2%	0.0%	0.1%	1.1%	3.5%	233	9425	2862	105	1092	185	142	304	116	179	7	11	165	531	P
/15/2018	1.6%	6 62	.2%	18.5%	6 0.7	%	6.7%	1.2%	0.7%	1.9%	0.7%	1.0%	0.1%	0.0%	1.0%	3.6%	250	9806	2911	111	1058	194	115	307	103	158	8	7	151	575	1
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Total	1 59	6 63	20%	18 69	. 05	0/	6.6%	1.2%	0.8%	2.0%	0.6%	0.8%	0.0%	0.1%	0.7%	3.5%	1644	68935	20225	571	7143	1310	825	2134	603	902	46	77	773	3708	

Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
	Cycle	+	Axle		Axle	Axle	Axle	Doubl	Doubl	Axle	Axle	Axle	Axle	ssifie	Volum
Bin Class	s	Traile	Long	Bus	6-Tire	Singl	Singl	е	е	Doubl	Multi	Multi	Multi	d	е
SurveyTotal	1644	68935	20225	571	7143	1319	825	2134	603	902	46	77	773	3798	108995
% Total	1.5%	63.2%	18.6%	0.5%	6.6%	1.2%	0.8%	2.0%	0.6%	0.8%	0.0%	0.1%	0.7%	3.5%	100%
Accumulated %	1.5%	64.8%	83.3%	83.8%	90.4%	91.6%	92.4%	94.3%	94.9%	95.7%	95.7%	95.8%	96.5%	100.0%	
Inv. Accum. %	98.5%	35.2%	16.7%	16.2%	9.6%	8.4%	7.6%	5.7%	5.1%	4.3%	4.3%	4.2%	3.5%	0.0%	



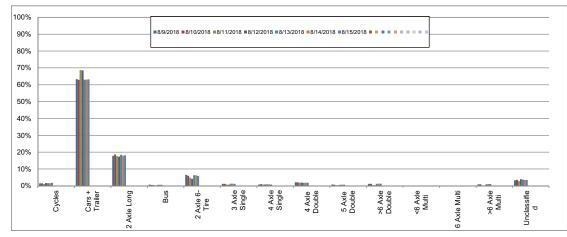
Classification Descriptions

Bin #	Bin Description
1	Motorcycles
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6	3-axle single units
7	4-axle single units
8	3-axle or 4-axle double unit
9	5-axle double unit
10	6-, 7-, or 8-axle double unit
11	5-axle multiple unit
12	6-axle multiple unit
13	7-or-more-axle multiple unit
14	Vehicles that could not be classified



Daily Class Bir	n Percen	tages													Daily C	lass B	in Volu	mes											
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
		Cars +	2 Axle	_	2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle			Cars +	2 Axle	_	2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle			Total
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volume
8/9/2018	1.5%	63.3%	17.8%	0.7%	6.5%	1.2%	0.8%	2.1%	0.8%	1.2%	0.0%	0.1%	0.9%	3.2%	215	9121	2560	97	936	169	110	304	112	171	7	9	126	467	14404
8/10/2018	1.4%	62.9%	18.6%	0.5%	5.9%	1.2%	1.0%	2.1%	0.6%	1.2%	0.1%	0.1%	0.9%	3.5%	214	9462	2802	81	893	184	147	311	92	176	8	12	135	530	15047
8/11/2018	1.1%	68.7%	17.6%	0.5%	4.7%	0.8%	0.7%	1.8%	0.4%	0.5%	0.0%	0.1%	0.3%	2.8%	142	9138	2344	63	631	107	98	244	49	62	3	11	35	372	13299
8/12/2018	1.7%	68.5%	17.3%	0.1%	4.1%	0.6%	0.8%	1.9%	0.4%	0.3%	0.0%	0.1%	0.2%	3.9%	241	9712	2454	18	579	90	113	270	52	48	5	11	24	553	14170
8/13/2018	1.5%	63.0%	18.4%	0.5%	6.3%	1.3%	0.9%	1.7%	0.6%	1.2%	0.0%	0.1%	0.9%	3.6%	211	8702	2536	75	869	173	122	241	88	162	4	11	124	500	13818
8/14/2018	1.5%	63.0%	17.9%	0.7%	6.2%	1.3%	0.9%	1.9%	0.7%	1.3%	0.1%	0.0%	1.1%	3.5%	208	8605	2447	101	844	172	127	258	101	174	9	3	145	475	13669
8/15/2018	1.8%	63.2%	18.0%	0.7%	5.9%	1.2%	0.8%	1.9%	0.7%	1.3%	0.0%	0.0%	1.1%	3.5%	250	8962	2558	93	830	171	116	263	94	180	4	7	153	496	14177
Total	1.5%	64.6%	18.0%	0.5%	5.7%	1.1%	0.8%	1.9%	0.6%	1.0%	0.0%	0.1%	0.8%	3.4%	1481	63702	17701	528	5582	1066	833	1891	588	973	40	64	742	3393	98584

Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Unclass	Tota
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volume
Survey Total	1481	63702	17701	528	5582	1066	833	1891	588	973	40	64	742	3393	9858
% Total	1.5%	64.6%	18.0%	0.5%	5.7%	1.1%	0.8%	1.9%	0.6%	1.0%	0.0%	0.1%	0.8%	3.4%	100%
Accumulated %	1.5%	66.1%	84.1%	84.6%	90.3%	91.4%	92.2%	94.1%	94.7%	95.7%	95.7%	95.8%	96.6%	100.0%	
Inv. Accum. %	98.5%	33.9%	15.9%	15.4%	9.7%	8.6%	7.8%	5.9%	5.3%	4.3%	4.3%	4.2%	3.4%	0.0%	

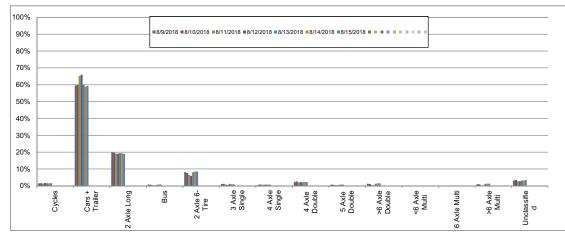


Bin #	Bin Description
1	Motorcycles
2	Passenger cars, including those with recreational trailers
3	2-axle pick-ups, vans, and RVs, including those with recreational trailers
4	Buses
5	2-axle, 6-tire single units
6	3-axle single units
7	4-axle single units
8	3-axle or 4-axle double unit
9	5-axle double unit
10	6-, 7-, or 8-axle double unit
11	5-axle multiple unit
12	6-axle multiple unit
13	7-or-more-axle multiple unit
14	Vehicles that could not be classified



ily Class Bin	Percen	tages													Daily C	Class B	in Vol	umes											i i
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Class	Cycles	Cars + Trailer	2 Axle Long	Bus	2 Axle 6-Tire	3 Axle Single	4 Axle Single	4 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Unclass ified	Cycles	Cars + Trailer	2 Axle Long	Bus	2 Axle 6-Tire	3 Axle Single	4 Axle Single	4 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Unclass ified	T Voli
8/9/2018	1.5%	59.6%	19.8%	0.8%	8.1%	1.1%	0.6%	2.3%	0.7%	1.3%	0.0%	0.1%	1.0%	3.2%	204	8225	2733	107	1119	146	78	322	97	179	5	13	133	436	13
8/10/2018	1.5%	59.8%	19.7%	0.6%	7.6%	1.1%	0.8%	2.6%	0.6%	1.1%	0.1%	0.1%	0.9%	3.5%	232	8987	2962	90	1143	171	118	388	87	160	11	16	136	519	15
8/11/2018	1.3%	65.3%	19.1%	0.4%	6.5%	0.8%	0.6%	2.0%	0.4%	0.4%	0.0%	0.1%	0.3%	2.7%	163	8227	2411	53	816	96	74	257	54	53	4	13	38	340	12
8/12/2018	1.6%	65.9%	19.1%	0.2%	6.0%	0.5%	0.6%	2.3%	0.3%	0.4%	0.0%	0.1%	0.2%	2.7%	201	8070	2337	28	731	65	79	276	40	44	3	9	25	335	12
8/13/2018	1.6%	59.9%	19.4%	0.6%	8.1%	1.2%	0.8%	2.1%	0.6%	1.2%	0.0%	0.1%	1.2%	3.2%	200	7618	2466	70	1032	156	106	272	76		5	13	152	402	12
8/14/2018	1.5%	58.9%	19.4%	0.8%	8.5%	1.0%	0.7%	2.3%		1.6%	0.1%		1.3%	3.2%	187	7550		105	1092	127	93	295	99	200	8	10	163	407	12
8/15/2018	1.6%	59.3%	19.0%	0.8%	8.5%	0.9%	0.7%	2.2%	0.7%	1.5%	0.0%	0.1%	1.2%	3.5%	214	7880	2520	106	1125	124	89	297	99	201	3	10	164	462	13
	_																												
																													<u> </u>
																													<u> </u>
Total	1.5%	61.2%	19.4%	0.6%	7.6%	1.0%	0.7%	2.3%	0.6%	1.1%	0.0%	0.1%	0.9%	3.1%	1401	56557	17910	559	7058	885	637	2107	552	987	39	84	811	2901	92

endee Data eann	, nai y														
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Unclass	Tota
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volume
Survey Total	1401	56557	17910	559	7058	885	637	2107	552	987	39	84	811	2901	92488
% Total	1.5%	61.2%	19.4%	0.6%	7.6%	1.0%	0.7%	2.3%	0.6%	1.1%	0.0%	0.1%	0.9%	3.1%	100%
Accumulated %	1.5%	62.7%	82.0%	82.6%	90.3%	91.2%	91.9%	94.2%	94.8%	95.9%	95.9%	96.0%	96.9%	100.0%	
Inv. Accum. %	98.5%	37.3%	18.0%	17.4%	9.7%	8.8%	8.1%	5.8%	5.2%	4.1%	4.1%	4.0%	3.1%	0.0%	

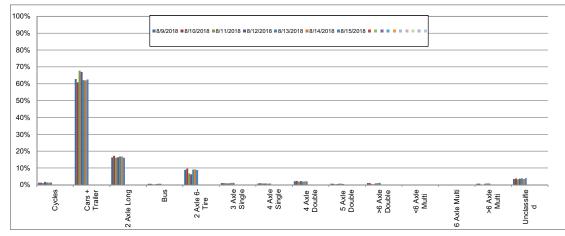


Bin #	Bin Description
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7	4-axle single units
8	3-axle or 4-axle double unit
9	5-axle double unit
10	6-, 7-, or 8-axle double unit
11	5-axle multiple unit
12	6-axle multiple unit
13	7-or-more-axle multiple unit
14	Vehicles that could not be classified



aily Class Bi	n Percen	tages													Daily C	lass B	Bin Volu	mes											
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle			Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle L		Tota
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volume
8/9/2018	1.3%	62.8%	16.2%	0.7%	8.8%	1.1%	0.8%	2.1%	0.7%	1.1%	0.0%	0.0%	0.7%	3.5%	214	10198	2636	107	1433	174	133	347	116	176	7	7	115	563	16226
8/10/2018	1.3%	60.8%	17.2%	0.6%	9.7%	0.9%	1.0%	2.3%	0.6%	0.9%	0.1%	0.1%	0.7%	3.8%	210	10054	2850	99	1597	157	159	381	104	157	14	21	114	626	16543
8/11/2018	1.1%	67.8%	16.1%	0.4%	6.8%	0.9%	0.8%	1.9%	0.3%	0.4%	0.0%	0.1%	0.2%	3.3%	157	9919	2353	54	994	139	114	273	50	59	2	8	33	481	14636
8/12/2018	1.8%	66.9%	16.4%	0.2%	6.2%	0.8%	0.8%	2.3%	0.4%	0.3%	0.1%	0.1%	0.1%	3.6%	274	10305	2521	31	953	118	120	347	67	48	10	18	22	559	15393
8/13/2018	1.4%	62.1%	16.8%	0.5%	9.1%	1.0%	0.8%	1.9%	0.7%	0.9%	0.0%	0.1%	0.7%	3.8%	211	9464	2563	82	1384	150	121	291	111	142	6	13	108	586	15232
8/14/2018	1.4%	61.9%	16.7%	0.7%	9.2%	1.1%	0.7%	2.0%	0.7%	1.1%	0.0%	0.1%	0.9%	3.5%	211	9390	2538	101	1389	174	103	309	102	170	6	11	131	529	15164
8/15/2018	1.4%	62.6%	16.1%	0.6%	8.8%	1.2%	0.8%	2.1%	0.5%	1.1%	0.0%	0.0%	0.8%	3.9%	217	9921	2558	93	1393	190	132	327	83	182	3	7	134	617	1585
Total	1.4%	63.5%	16.5%	0.5%	8.4%	1.0%	0.8%	2.1%	0.6%	0.9%	0.0%	0.1%	0.6%	3.6%	1494	69251	18019	567	9143	1102	882	2275	633	934	48	85	657	3961	10005

Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Unclass	Tota
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volume
Survey Total	1494	69251	18019	567	9143	1102	882	2275	633	934	48	85	657	3961	109051
% Total	1.4%	63.5%	16.5%	0.5%	8.4%	1.0%	0.8%	2.1%	0.6%	0.9%	0.0%	0.1%	0.6%	3.6%	100%
Accumulated %	1.4%	64.9%	81.4%	81.9%	90.3%	91.3%	92.1%	94.2%	94.8%	95.6%	95.7%	95.8%	96.4%	100.0%	
Inv. Accum. %	98.6%	35.1%	18.6%	18.1%	9.7%	8.7%	7.9%	5.8%	5.2%	4.4%	4.3%	4.2%	3.6%	0.0%	

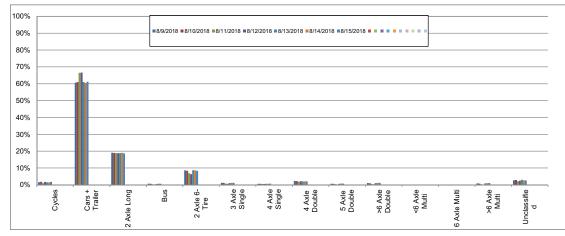


Bin #	Bin Description
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2	Passenger cars, including those with recreational trailers
3	2-axle pick-ups, vans, and RVs, including those with recreational trailers
4	Buses
5	2-axle, 6-tire single units
6	3-axle single units
7	4-axle single units
8	3-axle or 4-axle double unit
9	5-axle double unit
10	6-, 7-, or 8-axle double unit
11	5-axle multiple unit
12	6-axle multiple unit
13	7-or-more-axle multiple unit
14	Vehicles that could not be classified



aily Class Bi	n Percen	tages													Daily C	Class E	lin Volu	mes											
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle			Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle L		Tota
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volume
8/9/2018	1.5%	60.6%	19.0%	0.7%	8.6%	1.2%	0.6%	2.3%	0.6%	1.1%	0.0%	0.0%	0.9%	2.7%	238	9355	2937	108	1323	179	95	358	94	173	3	3	142	421	1542
8/10/2018	1.9%	61.0%	19.0%	0.6%	8.3%	1.2%	0.6%	2.3%	0.6%	0.9%	0.0%	0.0%	0.7%	2.9%	314	10272	3198	100	1400	202	101	383	107	146	7	8	124	489	1685
8/11/2018	1.0%	66.5%	18.9%	0.4%	6.9%	0.7%	0.6%	1.8%	0.4%	0.4%	0.0%	0.0%	0.3%	2.2%	138	9430	2675	50	978	94	78	257	54	54	4	7	47	307	1417
8/12/2018	1.7%	66.6%	18.8%	0.2%	6.3%	0.6%	0.5%	2.1%	0.3%	0.3%	0.0%	0.0%	0.2%	2.4%	231	8892	2505	33	840	77	71	280	35	38	3	4	21	321	1335
8/13/2018	1.4%	61.0%	18.9%	0.6%	8.7%	1.1%	0.6%	2.1%	0.6%	1.1%	0.0%	0.1%	0.9%	3.0%	206	8716	2699	79	1238	158	86	297	92	155	7	10	126	422	1429
8/14/2018	1.5%	60.5%	19.0%	0.7%	8.6%	1.2%	0.6%	2.1%	0.8%	1.2%	0.0%	0.1%	1.0%	2.7%	217	8729	2742	101	1237	167	93	296	119	176	5	11	149	397	1443
8/15/2018	1.7%	61.2%	18.6%	0.6%	8.3%	1.2%	0.7%	2.1%	0.7%	1.2%	0.0%	0.0%	1.1%	2.6%	265	9289	2828	97	1259	182	102	312	105	179	1	6	163	394	1518
																													_
Total	1.6%	62.4%	18.9%	0.5%	8.0%	1.0%	0.6%	2.1%	0.6%	0.9%	0.0%	0.0%	0.7%	2.7%	1609	64683	19584	568	8275	1059	626	2183	606	921	30	49	772	2751	10371

Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Unclass	Tota
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volume
Survey Total	1609	64683	19584	568	8275	1059	626	2183	606	921	30	49	772	2751	103716
% Total	1.6%	62.4%	18.9%	0.5%	8.0%	1.0%	0.6%	2.1%	0.6%	0.9%	0.0%	0.0%	0.7%	2.7%	100%
Accumulated %	1.6%	63.9%	82.8%	83.3%	91.3%	92.3%	92.9%	95.1%	95.6%	96.5%	96.6%	96.6%	97.3%	100.0%	
Inv. Accum. %	98.4%	36.1%	17.2%	16.7%	8.7%	7.7%	7.1%	4.9%	4.4%	3.5%	3.4%	3.4%	2.7%	0.0%	

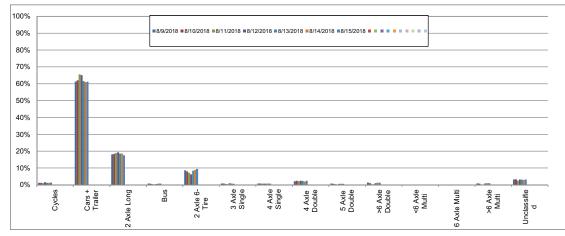


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12	6-axle multiple unit
13	7-or-more-axle multiple unit
14	Vehicles that could not be classified



Daily Class Bin	Percen	tages													Daily C	lass B	in Vol	umes											
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Class	Cycles	Cars + Trailer	2 Axle Long	Bus	2 Axle 6-Tire	3 Axle Single	4 Axle Single	4 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Unclass ified	Cycles	Cars + Trailer	2 Axle Long	Bus	2 Axle 6-Tire	3 Axle Single	4 Axle Single	4 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Jnclass ified	Tota Volume
8/9/2018	1.2%	61.3%	18.1%	0.8%	8.6%	0.8%	0.8%	2.2%	0.8%	1.4%	0.0%	0.1%	0.9%	3.1%	158	8058	2376	106	1126	108	103	283	103	185	6	9	116	412	13149
8/10/2018	1.2%	62.0%	18.4%	0.6%	7.9%	0.8%	0.8%	2.4%	0.6%	1.0%	0.1%	0.2%	0.7%	3.3%	162	8622	2556	83	1100	110	115	330	86	141	12	21	104	460	13902
8/11/2018	1.0%	65.5%	18.8%	0.5%	7.1%	0.5%	0.7%	2.2%		0.4%	0.0%	0.1%	0.2%	2.5%	126	8003	2301	57	869	61	82	263	53	44	6	9	29	310	12213
8/12/2018	1.5%	65.1%	19.3%	0.2%	6.2%	0.4%	0.7%	2.4%	0.3%	0.4%	0.0%	0.1%	0.2%	3.2%	195	8351	2481	28	794	46	90	309	34	48	5	12	24	413	12830
8/13/2018	1.2%		18.5%	0.6%	8.4%	1.0%	0.7%	2.3%	0.6%	1.1%	0.0%	0.1%	0.9%	3.1%	154	7827	2358	71	1066	123	87	297	80	141	3	13	113	388	1272
8/14/2018	1.2%		18.6%	0.8%	8.9%	0.8%	0.8%	2.0%		1.3%	0.0%	0.1%	1.0%	2.9%	154	7728	2351	99	1124	104	105	251	79	160	5	-	124	368	12660
8/15/2018	1.4%	61.2%	17.5%	0.8%	9.4%	0.6%	0.7%	2.4%	0.6%	1.3%	0.1%	0.1%	1.0%	3.1%	181	8011	2288	99	1236	81	89	311	76	170	8	12	125	411	13098
	-																												
Total	1.2%	62.5%	18.5%	0.6%	8.1%	0.7%	0.7%	2.3%	0.6%	1.0%	0.0%	0.1%	0.7%	3.0%	1130	56600	16711	543	7315	633	671	2044	511	889	45	84	635	2762	9057

Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Unclass	Tota
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volume
Survey Total	1130	56600	16711	543	7315	633	671	2044	511	889	45	84	635	2762	90573
% Total	1.2%	62.5%	18.5%	0.6%	8.1%	0.7%	0.7%	2.3%	0.6%	1.0%	0.0%	0.1%	0.7%	3.0%	100%
Accumulated %	1.2%	63.7%	82.2%	82.8%	90.9%	91.6%	92.3%	94.6%	95.1%	96.1%	96.2%	96.2%	97.0%	100.0%	
Inv. Accum. %	98.8%	36.3%	17.8%	17.2%	9.1%	8.4%	7.7%	5.4%	4.9%	3.9%	3.8%	3.8%	3.0%	0.0%	

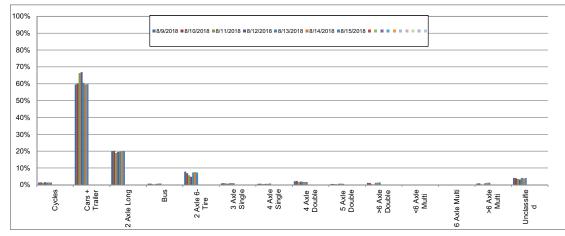


Bin #	Bin Description
1	Motorcycles
2	Passenger cars, including those with recreational trailers
3	2-axle pick-ups, vans, and RVs, including those with recreational trailers
4	Buses
5	2-axle, 6-tire single units
6	3-axle single units
7	4-axle single units
8	3-axle or 4-axle double unit
9	5-axle double unit
10	6-, 7-, or 8-axle double unit
11	5-axle multiple unit
12	6-axle multiple unit
13	7-or-more-axle multiple unit
14	Vehicles that could not be classified



Daily Class Bin	Percen	tages													Daily C	lass B	in Vol	umes											1
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	<u> </u>
Bin Class	Cycles	Cars + Trailer	2 Axle Long	Bus	2 Axle 6-Tire	3 Axle Single	4 Axle Single	4 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Unclass ified	Cycles	Cars + Trailer	2 Axle Long	Bus	2 Axle 6-Tire	3 Axle Single	4 Axle Single	4 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Inclass ified	Tota Volume
8/9/2018	1.4%	59.6%	20.1%	0.8%	7.8%	0.9%	0.6%	2.1%	0.5%	1.2%	0.0%	0.1%	0.8%	4.1%	183	7765	2623	98	1017	121	75	272	68	158	1	7	108	541	1303
8/10/2018	1.4%	60.3%	20.2%	0.7%	6.8%	1.0%	0.7%	2.3%	0.5%	1.1%	0.1%		0.9%	4.0%	206	8600	2883	95	976	140	100	331	71	156	8	14	125	568	14273
8/11/2018	1.1%	66.3%	19.0%	0.5%	5.6%	0.9%	0.4%	1.6%	0.4%	0.5%	0.0%	0.0%	0.3%	3.4%	139	8027	2297	57	678	107	51	188	46	55	2	6	38	414	1210
8/12/2018	1.6%		19.5%	0.2%	4.8%	0.6%	0.4%	2.0%	0.3%	0.2%	0.0%	0.1%	0.2%	3.2%	179	7556	2203	27	543	65	46	222	39	24	1	6	25	364	11300
8/13/2018	1.4%		19.7%	0.6%	7.4%	1.0%	0.6%	1.7%	0.7%	1.2%	0.0%	0.1%	1.0%	4.1%	166	7268		68	888	122	77	208		138	4	8	119	491	12000
8/14/2018	1.4%		19.9%	0.8%	7.5%	1.1%	0.6%	1.8%	0.7%	1.4%	0.0%	0.0%	1.2%	3.9%	175	7194	2406	95	906	129	77	217	90	167	5	6	145	468	
8/15/2018	1.4%	59.8%	20.0%	0.8%	7.3%	1.0%	0.8%	1.7%	0.6%	1.5%	0.0%	0.0%	1.3%	4.0%	175	7563	2529	98	919	123	99	211	77	185	1	6	160	511	1265
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Total	14%	61.7%	19.8%	0.6%	6.8%	0.9%	0.6%	1.9%	0.5%	1.0%	0.0%	0.1%	0.8%	3.8%	1223	53973	17304	538	5927	807	525	1649	471	883	22	53	720	3357	87452

Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Unclass	Tota
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volume
Survey Total	1223	53973	17304	538	5927	807	525	1649	471	883	22	53	720	3357	87452
% Total	1.4%	61.7%	19.8%	0.6%	6.8%	0.9%	0.6%	1.9%	0.5%	1.0%	0.0%	0.1%	0.8%	3.8%	100%
Accumulated %	1.4%	63.1%	82.9%	83.5%	90.3%	91.2%	91.8%	93.7%	94.2%	95.3%	95.3%	95.3%	96.2%	100.0%	
Inv. Accum. %	98.6%	36.9%	17.1%	16.5%	9.7%	8.8%	8.2%	6.3%	5.8%	4.7%	4.7%	4.7%	3.8%	0.0%	

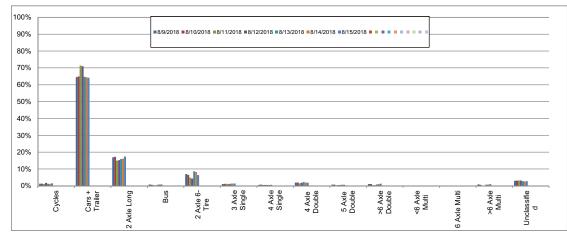


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10	6-, 7-, or 8-axle double unit
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14	Vehicles that could not be classified



aily Class Bir	n Percen	tages													Daily C	lass B	in Vol	umes											
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Class	Cycles	Cars + Trailer	2 Axle Long	Bus	2 Axle 6-Tire	3 Axle Single	4 Axle Single	4 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Unclass ified	Cycles	Cars + Trailer	2 Axle Long	Bus	2 Axle 6-Tire	3 Axle Single	4 Axle Single	4 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Inclass ified	Tota Volume
8/9/2018	1.3%	64.5%	16.9%	0.8%	7.0%	1.1%	0.7%	2.0%	0.7%	1.1%	0.0%	0.1%	0.9%	3.0%	172	8775	2299	110	945	151	89	267	101	153	5	7	116	407	13597
8/10/2018	1.3%	64.9%	17.2%	0.6%	6.5%	1.2%	0.7%	2.0%	0.7%	1.1%	0.1%	0.1%	0.6%	3.1%	180	9340	2478	91	932	167	104	283	97	159	9	13	88	446	14387
8/11/2018	1.1%	71.4%	14.9%	0.4%	4.9%	1.2%	0.6%	1.5%	0.3%	0.4%	0.0%	0.1%	0.2%	3.1%	134	9109	1903	56	620	151	72	187	40	54	5	8	20	401	12760
8/12/2018	1.7%	71.0%	15.2%	0.2%	4.3%	1.0%	0.6%	1.9%	0.4%	0.3%	0.0%	0.1%	0.2%	3.1%	234	9560	2047	32	577	130	76	250	51	42	5	12	21	424	1346
8/13/2018	1.2%	64.7%	15.9%	0.6%	8.6%	1.3%	0.5%	2.2%	0.5%	0.8%	0.0%	0.1%	0.7%	2.8%	162	8579	2103	86	1144	170	69	293	72		4	11	93	367	1326
8/14/2018	1.2%	64.4%	16.2%	0.9%	8.2%	1.4%	0.6%	1.9%	0.7%	1.1%	0.0%	0.0%	0.8%	2.6%	150	8340	2101	111	1060	182	72	252	88	139	3	5	107	340	12950
8/15/2018	1.5%	64.1%	17.4%	0.8%	6.4%	1.4%	0.6%	1.8%	0.7%	1.3%	0.1%	0.0%	1.0%	2.8%	201	8638	2350	106	864	191	84	249	89	174	8	6	137	377	13474
	_																												_
Total	1.3%	66.4%	16.3%	0.6%	6.5%	1.2%	0.6%	1.9%	0.6%	0.9%	0.0%	0.1%	0.6%	2.9%	1233	62341	15281	592	6142	1142	566	1781	538	829	39	62	582	2762	93890

Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Unclass	Tota
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volum
Survey Total	1233	62341	15281	592	6142	1142	566	1781	538	829	39	62	582	2762	9389
% Total	1.3%	66.4%	16.3%	0.6%	6.5%	1.2%	0.6%	1.9%	0.6%	0.9%	0.0%	0.1%	0.6%	2.9%	100%
Accumulated %	1.3%	67.7%	84.0%	84.6%	91.2%	92.4%	93.0%	94.9%	95.4%	96.3%	96.4%	96.4%	97.1%	100.0%	
Inv. Accum. %	98.7%	32.3%	16.0%	15.4%	8.8%	7.6%	7.0%	5.1%	4.6%	3.7%	3.6%	3.6%	2.9%	0.0%	

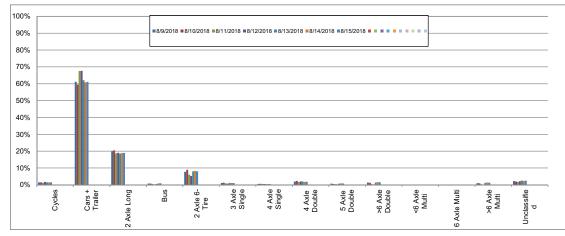


Bin #	Bin Description
1	Motorcycles
2	Passenger cars, including those with recreational trailers
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4	Buses
5	2-axle, 6-tire single units
6	3-axle single units
7	4-axle single units
8	3-axle or 4-axle double unit
9	5-axle double unit
10	6-, 7-, or 8-axle double unit
11	5-axle multiple unit
12	6-axle multiple unit
13	7-or-more-axle multiple unit
14	Vehicles that could not be classified



aily Class Bir	Percen	tages													Daily C	lass B	in Voli	umes											
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	_
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle U			Cars +	2 Axle	_	2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle		nclass	Tot
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volun
8/9/2018	1.4%	61.2%	19.9%	0.8%	7.8%	1.1%	0.5%	1.9%	0.7%	1.3%	0.0%	0.1%	1.0%	2.2%	195	8258	2678	113	1046	150	66	257	100	181	5	7	131	300	1348
8/10/2018	1.5%	59.5%	20.5%	0.7%	9.0%	1.3%	0.6%	2.3%	0.5%	1.2%	0.1%	0.1%	0.9%	1.9%	220	8953	3085	104	1351	192	92	350	78	177	9	17	136	292	1505
8/11/2018	1.2%	67.5%	18.6%	0.5%	6.0%	0.9%	0.6%	1.7%	0.4%	0.4%	0.0%	0.0%	0.3%	1.8%	153	8429	2325	62	754	109	70	209	50	51	4	2	43	223	1248
8/12/2018	1.8%	67.6%	19.0%	0.2%	5.3%	0.7%	0.5%	2.0%	0.3%	0.3%	0.0%	0.0%	0.2%	2.1%	207	7855	2208	26	617	81	56	234	34	31	3	1	21	245	1161
8/13/2018	1.4%	62.0%	18.5%	0.6%	8.0%	1.2%	0.5%	1.9%	0.8%	1.4%	0.0%	0.0%	1.2%	2.5%	176	7681	2296	69	993	146	56	238	93	174	4	5	143	306	1238
8/14/2018	1.4%	60.8%	18.9%	0.9%	8.3%	1.0%	0.5%	1.8%	0.9%	1.6%	0.0%	0.1%	1.4%	2.3%	178	7592	2363	108	1038	131	61	224	109	200	4	7	174	288	1247
8/15/2018	1.5%	61.1%	18.9%	0.9%	8.0%	1.0%	0.5%	1.8%	0.8%	1.6%	0.0%	0.1%	1.3%	2.5%	193	7999	2479	113	1053	137	61	238	104	203	1	9	171	333	1309
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	-																												
																													_
	_																												
Total	1.5%	62.7%	19.2%	0.7%	7.6%	1.0%	0.5%	1.9%	0.6%	1.1%	0.0%	0.1%	0.9%	2.2%	1322	56767	17434	595	6852	946	462	1750	568	1017	30	48	819	1987	9059

Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Unclass	Tota
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volume
Survey Total	1322	56767	17434	595	6852	946	462	1750	568	1017	30	48	819	1987	9059
% Total	1.5%	62.7%	19.2%	0.7%	7.6%	1.0%	0.5%	1.9%	0.6%	1.1%	0.0%	0.1%	0.9%	2.2%	100%
Accumulated %	1.5%	64.1%	83.4%	84.0%	91.6%	92.6%	93.1%	95.1%	95.7%	96.8%	96.8%	96.9%	97.8%	100.0%	
Inv. Accum. %	98.5%	35.9%	16.6%	16.0%	8.4%	7.4%	6.9%	4.9%	4.3%	3.2%	3.2%	3.1%	2.2%	0.0%	

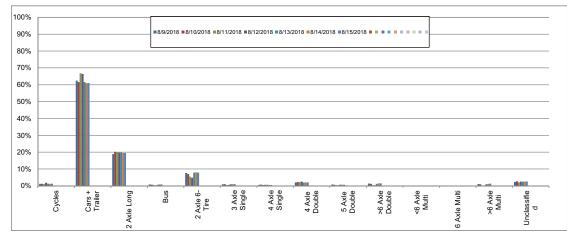


Bin #	Bin Description
1	Motorcycles
2	Passenger cars, including those with recreational trailers
3	2-axle pick-ups, vans, and RVs, including those with recreational trailers
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7	4-axle single units
8	3-axle or 4-axle double unit
9	5-axle double unit
10	6-, 7-, or 8-axle double unit
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ily Class Bir	n Percen	ntages													Daily C	lass B	in Volu	umes											
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Class	Cycles	Cars + Trailer	2 Axle Long	Bus	2 Axle 6-Tire	3 Axle Single	4 Axle Single	4 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Unclass ified	Cycles	Cars + Trailer	2 Axle Long	Bus	2 Axle 6-Tire	3 Axle Single	4 Axle Single	4 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle U Multi	Inclass ified	To Volui
8/9/2018	1.2%	62.4%	18.8%	0.8%	7.7%	0.9%	0.6%	2.1%	0.8%	1.4%	0.0%	0.0%	1.0%	2.3%	156	8129	2453	105	999	113	76	271	104	181	3	2	136	301	130
8/10/2018	1.2%	61.5%	20.2%	0.7%	7.0%	1.0%	0.7%	2.2%	0.6%	1.2%	0.1%	0.1%	0.9%	2.7%	165	8397	2753	100	952	131	91	303	87	159	11	12	118	366	136
8/11/2018	1.0%	66.8%	19.9%	0.5%	5.5%	0.5%	0.5%	2.2%	0.4%	0.4%	0.1%	0.0%	0.3%	2.0%	114	8011	2383	59	659	60	60	264	53	49	8	5	31	240	119
8/12/2018	1.8%	66.5%	19.7%	0.2%	4.8%	0.5%	0.6%	2.5%	0.3%	0.3%	0.0%	0.1%	0.2%	2.5%	227	8353	2471	30	603	60	77	313	40	37	6	9	28	314	125
8/13/2018	1.3%	61.6%	19.7%	0.6%	7.8%	1.0%	0.6%	1.9%	0.7%	1.2%	0.0%	0.1%	1.0%	2.5%	158	7715	2464	79	975	122	77	240	89	146	2	14	128	319	125
8/14/2018	1.2%	60.9%	19.5%	0.9%	8.0%	1.0%	0.5%	2.1%	0.7%	1.4%	0.0%	0.0%	1.1%	2.5%	145	7451	2389	107	980	122	64	252	91	176	2	5	139	308	122
8/15/2018	1.3%	61.0%	19.5%	0.8%	7.8%	0.9%	0.5%	2.0%	0.6%	1.4%	0.0%	0.1%	1.3%	2.7%	164	7772	2480	105	997	121	69	255	82	184	3	11	160	338	127
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																											_		_
Total	1.3%	62.9%	19.6%	0.7%	6.9%	0.8%	0.6%	2.1%	0.6%	1.1%	0.0%	0.1%	0.8%	2.5%	1129	55828	17393	585	6165	729	514	1898	546	932	35	58	740	2186	887

Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Unclass	Tota
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volume
Survey Total	1129	55828	17393	585	6165	729	514	1898	546	932	35	58	740	2186	88738
% Total	1.3%	62.9%	19.6%	0.7%	6.9%	0.8%	0.6%	2.1%	0.6%	1.1%	0.0%	0.1%	0.8%	2.5%	100%
Accumulated %	1.3%	64.2%	83.8%	84.4%	91.4%	92.2%	92.8%	94.9%	95.5%	96.6%	96.6%	96.7%	97.5%	100.0%	
Inv. Accum. %	98.7%	35.8%	16.2%	15.6%	8.6%	7.8%	7.2%	5.1%	4.5%	3.4%	3.4%	3.3%	2.5%	0.0%	

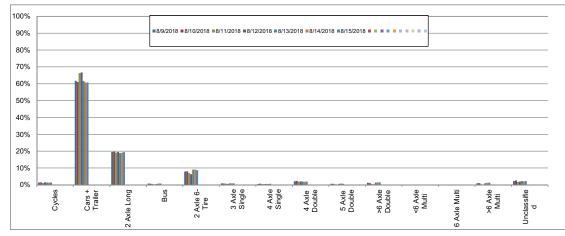


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14	Vehicles that could not be classified



aily Class Bir	n Percen	tages													Daily C	lass B	in Vol	umes											
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1	2	3	4	5	6	7	8	9	10	11	12	13	14	
Bin Class	Cycles	Cars + Trailer	2 Axle Long	Bus	2 Axle 6-Tire	3 Axle Single	4 Axle Single	4 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle Multi	Unclass ified	Cycles	Cars + Trailer	2 Axle Long	Bus	2 Axle 6-Tire	3 Axle Single	4 Axle Single	4 Axle Double	5 Axle Double	>6 Axle Double	<6 Axle Multi	6 Axle Multi	>6 Axle U Multi	Inclass ified	Total Volume
8/9/2018	1.4%	61.7%	19.5%	0.8%	8.0%	0.9%	0.5%	2.1%	0.6%	1.2%	0.0%	0.0%	1.0%	2.2%	185	8435	2661	113	1089	119	65	290	84	170	3	4	135	307	13660
8/10/2018	1.5%	61.1%	19.7%	0.6%	8.0%	0.8%	0.7%	2.3%	0.6%	1.0%	0.0%	0.1%	1.0%	2.6%	222	8973	2895	91	1174	121	106	338	86	150	5	12	140	377	14690
8/11/2018	1.1%	66.4%	19.1%	0.5%	7.1%	0.7%	0.4%	1.9%	0.3%	0.4%	0.0%	0.0%	0.3%	1.8%	140	8264	2377	63	886	86	49	231	40	52	2	2	43	220	12455
8/12/2018	1.5%		19.5%	0.3%	6.4%	0.5%	0.4%	2.0%	0.2%	0.3%	0.0%	0.0%	0.2%	1.9%	175	7721	2264	29		62	48	236	27	34	2	4	23	221	11582
8/13/2018	1.4%	61.5%	18.9%	0.5%	9.1%		0.5%	2.0%	0.6%	1.4%	0.0%	0.1%	1.0%	2.2%	170	7662	2352	66	1132	108	61	244	71		4	7	119	280	12450
8/14/2018		60.7%		0.8%	9.1%		0.5%	1.8%	0.8%	1.5%		0.0%	1.2%	2.2%	167	7601	2397	101	1141	107	65	228	102		4	4	148	273	12525
8/15/2018	1.3%	60.7%	19.4%	0.8%	8.7%	0.9%	0.6%	1.9%	0.6%	1.5%	0.0%	0.0%	1.3%	2.2%	176	7981	2555	110	1141	113	76	256	84	193	2	5	172	290	13154
Total	1.4%	62.6%	19.3%	0.6%	8.1%	0.8%	0.5%	2.0%	0.5%	1.1%	0.0%	0.0%	0.9%	2.2%	1235	56637	17501	573	7299	716	470	1823	494	960	22	38	780	1968	90516

endee Data eann															
Bin #	1	2	3	4	5	6	7	8	9	10	11	12	13	14	1
		Cars +	2 Axle		2 Axle	3 Axle	4 Axle	4 Axle	5 Axle	>6 Axle	<6 Axle	6 Axle	>6 Axle	Unclass	Tota
Bin Class	Cycles	Trailer	Long	Bus	6-Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	ified	Volume
Survey Total	1235	56637	17501	573	7299	716	470	1823	494	960	22	38	780	1968	9051
% Total	1.4%	62.6%	19.3%	0.6%	8.1%	0.8%	0.5%	2.0%	0.5%	1.1%	0.0%	0.0%	0.9%	2.2%	100%
Accumulated %	1.4%	63.9%	83.3%	83.9%	92.0%	92.8%	93.3%	95.3%	95.8%	96.9%	96.9%	97.0%	97.8%	100.0%	
Inv. Accum. %	98.6%	36.1%	16.7%	16.1%	8.0%	7.2%	6.7%	4.7%	4.2%	3.1%	3.1%	3.0%	2.2%	0.0%	



Bin #	Bin Description
1	Motorcycles
2	Passenger cars, including those with recreational trailers
3	2-axle pick-ups, vans, and RVs, including those with recreational trailers
4	Buses
5	2-axle, 6-tire single units
6	3-axle single units
7	4-axle single units
8	3-axle or 4-axle double unit
9	5-axle double unit
10	6-, 7-, or 8-axle double unit
11	5-axle multiple unit
12	6-axle multiple unit
13	7-or-more-axle multiple unit
14	Vehicles that could not be classified

APPENDIX D

VEHICLE COLLISION DATA





ICBC Collision Data

City	Crash Type	Location	Year
LADYSMITH	Casualty	GROUHEL RD & TRANS-CANADA HWY & TURNING LANE	2011
LADYSMITH	Casualty	GROUHEL RD & TRANS-CANADA HWY & TURNING LANE	2012
LADYSMITH	Casualty	GROUHEL RD & TRANS-CANADA HWY & TURNING LANE	2014
LADYSMITH	Casualty	GROUHEL RD & TRANS-CANADA HWY & TURNING LANE	2015
LADYSMITH	PDO	GROUHEL RD & TRANS-CANADA HWY & TURNING LANE	2011
LADYSMITH	PDO	GROUHEL RD & TRANS-CANADA HWY & TURNING LANE	2012
LADYSMITH	PDO	GROUHEL RD & TRANS-CANADA HWY & TURNING LANE	2013
LADYSMITH	PDO	GROUHEL RD & TRANS-CANADA HWY & TURNING LANE	2014
LADYSMITH	PDO	GROUHEL RD & TRANS-CANADA HWY & TURNING LANE	2015
LADYSMITH	Casualty	1ST AVE & ESPLANADE & LUDLOW RD & TRANS-CANADA HWY	2011
LADYSMITH	Casualty	1ST AVE & ESPLANADE & LUDLOW RD & TRANS-CANADA HWY	2012
LADYSMITH	Casualty	1ST AVE & ESPLANADE & LUDLOW RD & TRANS-CANADA HWY	2013
LADYSMITH	Casualty	1ST AVE & ESPLANADE & LUDLOW RD & TRANS-CANADA HWY	2014
LADYSMITH	Casualty	1ST AVE & ESPLANADE & LUDLOW RD & TRANS-CANADA HWY	2015
LADYSMITH	PDO	1ST AVE & ESPLANADE & LUDLOW RD & TRANS-CANADA HWY	2011
LADYSMITH	PDO	1ST AVE & ESPLANADE & LUDLOW RD & TRANS-CANADA HWY	2012
LADYSMITH	PDO	1ST AVE & ESPLANADE & LUDLOW RD & TRANS-CANADA HWY	2013
LADYSMITH	PDO	1ST AVE & ESPLANADE & LUDLOW RD & TRANS-CANADA HWY	2014
LADYSMITH	PDO	1ST AVE & ESPLANADE & LUDLOW RD & TRANS-CANADA HWY	2015
LADYSMITH	Casualty	ESPLANADE & ROBERTS ST & TRANSFER BEACH BLVD & TURNING LANE	2012
LADYSMITH	Casualty	ESPLANADE & ROBERTS ST & TRANSFER BEACH BLVD & TURNING LANE	2013
LADYSMITH	Casualty	ESPLANADE & ROBERTS ST & TRANSFER BEACH BLVD & TURNING LANE	2014
LADYSMITH	Casualty	ESPLANADE & ROBERTS ST & TRANSFER BEACH BLVD & TURNING LANE	2015
LADYSMITH	PDO	ESPLANADE & ROBERTS ST & TRANSFER BEACH BLVD & TURNING LANE	2011
LADYSMITH	PDO	ESPLANADE & ROBERTS ST & TRANSFER BEACH BLVD & TURNING LANE	2012
LADYSMITH	PDO	ESPLANADE & ROBERTS ST & TRANSFER BEACH BLVD & TURNING LANE	2014
LADYSMITH	PDO	ESPLANADE & ROBERTS ST & TRANSFER BEACH BLVD & TURNING LANE	2015
LADYSMITH	Casualty	N DAVIS RD & N DAVIS RD OFFRAMP & N DAVIS RD ONRAMP & TRANS-CANADA	2011
LADYSMITH	Casualty	N DAVIS RD & N DAVIS RD OFFRAMP & N DAVIS RD ONRAMP & TRANS-CANADA	2012
LADYSMITH	Casualty	N DAVIS RD & N DAVIS RD OFFRAMP & N DAVIS RD ONRAMP & TRANS-CANADA	2013
LADYSMITH	Casualty	N DAVIS RD & N DAVIS RD OFFRAMP & N DAVIS RD ONRAMP & TRANS-CANADA	2014



ICBC Collision Data

City	Crash Type	Location	Year
LADYSMITH	Casualty	N DAVIS RD & N DAVIS RD OFFRAMP & N DAVIS RD ONRAMP & TRANS-CANADA	2015
LADYSMITH	PDO	N DAVIS RD & N DAVIS RD OFFRAMP & N DAVIS RD ONRAMP & TRANS-CANADA	2011
LADYSMITH	PDO	N DAVIS RD & N DAVIS RD OFFRAMP & N DAVIS RD ONRAMP & TRANS-CANADA	2012
LADYSMITH	PDO	N DAVIS RD & N DAVIS RD OFFRAMP & N DAVIS RD ONRAMP & TRANS-CANADA	2013
LADYSMITH	PDO	N DAVIS RD & N DAVIS RD OFFRAMP & N DAVIS RD ONRAMP & TRANS-CANADA	2014
LADYSMITH	PDO	N DAVIS RD & N DAVIS RD OFFRAMP & N DAVIS RD ONRAMP & TRANS-CANADA	2015
LADYSMITH	Casualty	EDGELOW RD & EDGELOW RD S & TRANS-CANADA HWY	2011
LADYSMITH	Casualty	EDGELOW RD & EDGELOW RD S & TRANS-CANADA HWY	2012
LADYSMITH	Casualty	EDGELOW RD & EDGELOW RD S & TRANS-CANADA HWY	2013
LADYSMITH	PDO	EDGELOW RD & EDGELOW RD S & TRANS-CANADA HWY	2012
LADYSMITH	PDO	EDGELOW RD & EDGELOW RD S & TRANS-CANADA HWY	2013
LADYSMITH	PDO	EDGELOW RD & EDGELOW RD S & TRANS-CANADA HWY	2015

BINNIE

CLSN YEAR	TOTAL INJ	TOTAL KLD	SPEED LIM	LOCN_TYPE	ROAD SURF	WEATHER	DIAGRAM	PREACTN1 Description	PREACTN2 Description	TYPE2ND1 Description	CONTRB11 Description	CONTRB12 Description	VE
2012	1	0	90	At intersection	Dry	Clear	Left turn rear end	Going straight ahead	Swerving	Motorcycle	Unknown	Not applicable	۱
2012	5	0	90	At intersection	Wet	Raining	Rear end	Slowing or stopping	Going straight ahead	Other motor vehicle	Exceeding speed limit	Road condition (ice,snow,slush)	٢
2012	1	0	90	Public Driveway	Snow	Snowing/sleet	Off road right	Spinning		Ran off/left roadway	Road condition (ice,snow,slush)	Driving too fast for condition	١
2012	2	0	70	At intersection	Dry	Clear	Intersection 90'	Making left turn	Going straight ahead	Other motor vehicle	Failing to yield right of way	Not applicable	
2012	1	0	90	Btwn intersection/exchs	Wet	Cloudy	Off road left	Swerving		Ran off/left roadway	Wild animal	Not applicable	٢
2012	2	0	90	Btwn intersection/exchs	Slush	Snowing/sleet	Off road left	Going straight ahead		Curbing	Road condition (ice,snow,slush)	Not applicable	S
2012	1	0	90	At intersection	Wet	Raining	Rear end	Slowing or stopping	Going straight ahead	Other motor vehicle	Unknown	Not applicable	٢
2012	1	0	90	Btwn intersection/exchs	Wet	Clear	Other	Swerving		Other	Exceeding speed limit	Wild animal	S
2013	1	0	90	At intersection	Dry	Clear	Rear end	Stopped in traffic	Going straight ahead	Other motor vehicle	Not applicable	Not applicable	١
2013	1	0	90	At intersection	Dry	Clear	Rear end	Going straight ahead	Stopped in traffic	Other motor vehicle	Driver inattentive	Following too closely	١
2013	1	0	90	Btwn intersection/exchs	Dry	Clear	Off road left	Yaw		Guard rail/traffic barrier	Tires-failure/inadequate	Not applicable	٢
2013	1	0	30	Entr/acceleration lane	Wet	Raining	Rear end	Slowing or stopping	Slowing or stopping	Other motor vehicle	Road condition (ice,snow,slush)	Weather (fog,sleet,rain,snow)	Un
2013	4	0	70	At intersection	Dry	Cloudy	Rear end	Going straight ahead	Going straight ahead	Other motor vehicle	Sudden loss of consciousness	Illness**	S
2014	6	0	90	At intersection	Wet	Raining	Rear end	Slowing or stopping	Going straight ahead	Other motor vehicle	Not applicable	Not applicable	S
2014	1	0	70	At intersection	Dry	Cloudy	Rear end	Stopped in traffic	Going straight ahead	Other motor vehicle	Driver inattentive	Not applicable	ſ
2014	1	0	70	At intersection	Dry	Clear	Other	Going straight ahead		Curbing	Driver inattentive	Not applicable	٦
2014	2	0	70	At intersection	Dry	Clear	Rear end	Slowing or stopping	Going straight ahead	Other motor vehicle	Not applicable	Not applicable	٦
2014	6	0	70	At intersection	Dry	Clear	Other	Making left turn	Going straight ahead	Other motor vehicle	Improper turning	Not applicable	S
2014	1	0	70	Btwn intersection/exchs	Wet	Raining	Rear end	Stopped in traffic	Stopped in traffic	Other motor vehicle	Following too closely	Not applicable	٢
2014	4	0	70	At intersection	Dry	Clear	Left turn 90'	Making left turn	Going straight ahead	Other motor vehicle	Driver error/confusion	Failing to yield right of way	٢
2014	4	0	90	Btwn intersection/exchs	Dry	Clear	Rear end	Going straight ahead	Going straight ahead	Other motor vehicle	Driver inattentive	Following too closely	S
2015	1	0	90	Btwn intersection/exchs	Dry	Clear	Rear end	Avoiding object on road	Avoiding object on road	Other motor vehicle	Driver internal/ external distr	Obstruction/debris on road	٦
2015	1	0	90	Btwn intersection/exchs	Wet	Raining	Unknown	Going straight ahead		Animal	Wild animal	Weather (fog,sleet,rain,snow)	5
2015	2	0	90	Entr/acceleration lane	Dry	Clear	Rear end	Going straight ahead	Going straight ahead	Other motor vehicle	Driver error/confusion	Not applicable	٢
2015	1	0	70	At intersection	Dry	Clear	Left turn head on	Changing lanes		Over turned	Obstruction/debris on road	Tires-failure/inadequate	S
2015	1	0	70	Entrance intersection	Dry	Clear	Rear end	Starting from traffic	Starting from traffic	Other motor vehicle	Driver inattentive	Not applicable	S
2015	1	0	70	At intersection	Wet	Clear	Side swipe	Going straight ahead	Making left turn	Other motor vehicle	Not applicable	Not applicable	S

3 - **BINNIE** File No. 18-0628-05 Appendix D - Vehicle Collision Data

VEHDIR1	VEHDIR2	ON	AT
North	North	1 HWY	THICKE / EDGELOW
North	North	1 HWY	EDGELOW RD
North		1 HWY	EDGELOW RD
East	South	1 HWY	GROUHEL
North		1 HWY	BAKER
South		1 HWY	WESTDOWNE RD
North	North	1 HWY	EDGELOW RD
South		1 HWY	EDGELOW RD
North	North	1 HWY	EDGELOW RD N
North	North	1 HWY	EDGELOW
North		1 HWY	BAKER RD
Unknown	Unknown	CHEMAINUS RD	N. DAVIS RD
South	South	1 HWY	1ST
South	South	1 HWY	DAVIS RD N
North	North	1 HWY	ROBERTS ST
North		1 HWY	ROBERTS ST
North	North	1 HWY	TRANSFER BEACH BLVD
South	South	1 HWY	GROUHEL RD
North	North	1 HWY	LUDLOW RD
North	South	1 HWY	1ST
South	South	1 HWY	N DAVIS
North	North	1 HWY	S. DAVIS RD
South		1 HWY	N. DAVIS RD
North	North	1 HWY	CHEMAINUS RD(N DAVIS)
South		1 HWY	LUDLOW RD
South	South	1 HWY	1ST AVE
South	North	1 HWY	ROBERTS ST

BINNIE

CLSN YEAR	TOTAL INJ	TOTAL KLD	SPEED LIM	LOCN_TYPE	ROAD SURF	WEATHER	DIAGRAM	PREACTN1 Description	PREACTN2 Description	TYPE2ND1 Description	CONTRB11 Description	CONTRB12 Description	VE
2015	1	0	90	Btwn intersection/exchs	Dry	Clear	Rear end	Merging	Merging	Other motor vehicle	Insufficient traffic control	Not applicable	S
2015	1	0	90	Btwn intersection/exchs	Wet	Clear	Overtaking	Swerving	Going straight ahead	Other motor vehicle	Driving too fast for condition	Cutting in	5
2015	1	0	90	Btwn intersection/exchs	Dry	Clear	Off road left	Unknown		Unknown	Unknown	Not applicable	S
2016	1	0	90	Btwn intersection/exchs	Wet	Raining	Off road right	Going straight ahead		Building/wall	Driver inattentive	Road condition (ice,snow,slush)	Ur
2016	1	0	90	At intersection	Dry	Clear	Rear end	Going straight ahead	Going straight ahead	Other motor vehicle	Driver inattentive	Not applicable	I
2016	1	0	90	Btwn intersection/exchs	Ice	Raining	Unknown	Going straight ahead		Building/wall	Driver inattentive	Road condition (ice,snow,slush)	I
2016	1	0	90	At intersection	Dry	Cloudy	Rear end	Unknown	Going straight ahead	Other motor vehicle	Driver internal/ external distr	Driver inattentive	9
2016	1	0	90	Btwn intersection/exchs	Ice	Cloudy	Off road left	Going straight ahead		Guard rail/traffic barrier	Road condition (ice,snow,slush)	Not applicable	I
2016	2	0	70	At intersection	Dry	Clear	Left turn 90'	Going straight ahead	Making left turn	Other motor vehicle	Not applicable	Not applicable	9
2016	1	0	70	At intersection	Dry	Clear	Left turn 90'	Going straight ahead	Making left turn	Other motor vehicle	Unknown	Not applicable	I
2016	2	0	70	At intersection	Dry	Clear	Head on	Making right turn	Going straight ahead	Other motor vehicle	Driver inattentive	Failing to yield right of way	ſ
2016	2	0	70	At intersection	Dry	Cloudy	Rear end	Stopped in traffic	Stopped in traffic	Other motor vehicle	Not applicable	Not applicable	I
2016	1	0	90	Btwn intersection/exchs	Wet	Raining	Off road right	Going straight ahead		Guard rail/traffic barrier	Driver inattentive	Driving too fast for condition	ſ
2016	3	0	70	At intersection	Wet	Cloudy	Intersection 90'	Going straight ahead	Making left turn	Other motor vehicle	Not applicable	Not applicable	5
2016	1	0	70	Btwn intersection/exchs	Ice	Clear	Unknown	Going straight ahead		Raised traffic island	Driver inattentive	Driving too fast for condition	9
2016	1	0	70	At intersection	Wet	Cloudy	Other	Going straight ahead	Going straight ahead	Other motor vehicle	Not applicable	Not applicable	9
2016	1	0	70	At intersection	Dry	Cloudy	Intersection 90'	Making right turn	Going straight ahead	Other motor vehicle	Failing to yield right of way	Not applicable	
2016	1	0	90	Btwn intersection/exchs	Dry	Clear	Rear end	Stopped in traffic	Going straight ahead	Other motor vehicle	Not applicable	Not applicable	9
2016	2	0	90	Btwn intersection/exchs	Wet	Raining	Off road left	Changing lanes		Ran off/left roadway	Road condition (ice,snow,slush)	Driving too fast for condition	9
2016	1	0	70	At intersection	Wet	Raining	Rear end	Slowing or stopping	Slowing or stopping	Other motor vehicle	Road condition (ice,snow,slush)	Not applicable	9
2016	3	0	90	At intersection	Wet	Clear	Intersection 90'	Going straight ahead	Making left turn	Other motor vehicle	Driver inattentive	Glare-sunlight	Ś

4 - *BINNIE* File No. 18-0628-05 Appendix D - Vehicle Collision Data

VEHDIR1	VEHDIR2	ON	АТ
South	South	1 HWY	DAVIS RD N
South	South	1 HWY	N DAVIS RD
South		1 HWY	WESTDOWNE RD
Unknown		1 HWY	WESTDOWNE RD
North	North	1 HWY	EDGELOW RD
North		1 HWY	S. DAVIS RD
South	South	1 HWY	N DAVIS RD
North		HOLLAND CREEK BRIDGE	1 HWY
South	West	1 HWY	ROBERTS ST
North	South	1 HWY	LUDLOW
North	South	CHEMAINUS RD	1ST AVE
North	North	1 HWY	1ST
North		1 HWY	GROUHEL RD
South	North	1 HWY	GROUHEL RD
South		1 HWY	KITCHENER ST
South	East	1 HWY	ROBERTS ST
East	South	ESPLANADE	1 HWY
South	South	1 HWY	N DAVIS
South		1 HWY	N DAVIS RD
South	South	1 HWY	S DAVIS RD
South	East	1 HWY	EDGELOW RD

APPENDIX E

SYNCHRO ANALYSIS RESULTS



1: Hwy 1 & Grouhel Rd

	≯	\mathbf{r}	1	Ť	Ļ				
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	¥		1	^	^	1			
Traffic Volume (veh/h)	23	23	16	902	922	19			
Future Volume (Veh/h)	23	23	16	902	922	19			
Sign Control	Stop			Free	Free				
Grade	0%			0%	0%				
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92			
Hourly flow rate (vph)	25	25	17	980	1002	21			
Pedestrians									
_ane Width (m)									
Walking Speed (m/s)									
Percent Blockage									
Right turn flare (veh)									
Median type				None	None				
Median storage veh)				110110	10110				
Upstream signal (m)									
oX, platoon unblocked									
vC, conflicting volume	1526	501	1002						
vC1, stage 1 conf vol	1020	001	1002						
vC2, stage 2 conf vol									
vCu, unblocked vol	1526	501	1002						
C, single (s)	6.8	6.9	4.1						
C, 2 stage (s)	0.0	0.0							
F (s)	3.5	3.3	2.2						
p0 queue free %	76	95	98						
cM capacity (veh/h)	106	515	687						
					0.5.4	05.0	00.0		
Direction, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3		
Volume Total	50	17	490	490	501	501	21		
Volume Left	25	17	0	0	0	0	0		
Volume Right	25	0	0	0	0	0	21		
cSH	175	687	1700	1700	1700	1700	1700		
Volume to Capacity	0.28	0.02	0.29	0.29	0.29	0.29	0.01		
Queue Length 95th (m)	8.5	0.6	0.0	0.0	0.0	0.0	0.0		
Control Delay (s)	33.5	10.4	0.0	0.0	0.0	0.0	0.0		
_ane LOS	D	В							
Approach Delay (s)	33.5	0.2			0.0				
Approach LOS	D								
Intersection Summary									
Average Delay			0.9						
Intersection Capacity Utilization			35.5%	IC	U Level of	Service		А	
Analysis Period (min)			15						

2: Hwy 1 & 1st Ave/Ludlow Rd

	≯	+	*	4	ł	*	•	1	1	*	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	1.		ሻ	↑	1	ሻ	- 44	1	ሻ	- † †	1
Traffic Volume (vph)	225	39	48	32	22	29	18	670	51	44	765	146
Future Volume (vph)	225	39	48	32	22	29	18	670	51	44	765	146
Satd. Flow (prot)	1789	1713	0	1789	1883	1601	1789	3579	1601	1789	3579	1601
Flt Permitted	0.742			0.696			0.339			0.257		
Satd. Flow (perm)	1398	1713	0	1308	1883	1601	638	3579	1601	484	3579	1601
Satd. Flow (RTOR)		52				185			112			159
Lane Group Flow (vph)	245	94	0	35	24	32	20	728	55	48	832	159
Turn Type	Perm	NA		Perm	NA	Free	Perm	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases	4			8		Free	2		2	6		6
Total Split (s)	22.0	22.0		24.2	24.2		51.4	51.4	51.4	20.7	52.1	52.1
Total Lost Time (s)	7.0	7.0		7.2	7.2		6.4	6.4	6.4	5.7	6.4	6.4
Act Effct Green (s)	15.5	15.5		15.2	15.2	60.6	24.8	24.8	24.8	32.1	31.3	31.3
Actuated g/C Ratio	0.26	0.26		0.25	0.25	1.00	0.41	0.41	0.41	0.53	0.52	0.52
v/c Ratio	0.69	0.20		0.11	0.05	0.02	0.08	0.50	0.08	0.12	0.45	0.18
Control Delay	36.9	13.7		23.3	22.6	0.0	13.4	15.3	0.6	6.5	9.4	1.7
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.9	13.7		23.3	22.6	0.0	13.4	15.3	0.6	6.5	9.4	1.7
LOS	D	В		С	С	А	В	В	А	А	А	A
Approach Delay		30.5			14.9			14.2			8.1	
Approach LOS		С			В			В			А	
Queue Length 50th (m)	26.8	3.9		3.3	2.2	0.0	1.5	34.8	0.0	2.3	27.0	0.0
Queue Length 95th (m)	#71.6	16.2		11.3	8.5	0.0	5.3	49.4	1.1	5.6	36.7	5.8
Internal Link Dist (m)		181.2			128.9			325.9			168.6	
Turn Bay Length (m)	90.0			65.0			140.0		130.0	160.0		
Base Capacity (vph)	408	537		378	544	1601	488	2737	1250	588	3473	1558
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.60	0.18		0.09	0.04	0.02	0.04	0.27	0.04	0.08	0.24	0.10
Intersection Summary												
Cycle Length: 96.3												
Actuated Cycle Length: 60.6												
Control Type: Actuated-Uncoor	rdinated											
Maximum v/c Ratio: 0.69												
Intersection Signal Delay: 13.9				In	tersection	LOS: B						
Intersection Capacity Utilization	า 66.9%			IC	U Level of	Service C	;					
Analysis Period (min) 15												
# 95th percentile volume exce	eeds capacit	y, queue n	nay be lon	ger.								

Queue shown is maximum after two cycles.

Splits and Phases: 2: Hwy 1 & 1st Ave/Ludlow Rd

Ø1		 Ø4	
20.7 s	51.4 s	22 s	
↓ _{Ø6}		₹ø8	
52.1 s		24.2 s	

3: Hwy 1 & Roberts St

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Lane Configurations		र्स	1			1	ሻ	- † †	1	ሻ	≜1 ≱	
Traffic Volume (vph)	51	20	51	8	11	13	93	739	22	21	817	1
Future Volume (vph)	51	20	51	8	11	13	93	739	22	21	817	1
Satd. Flow (prot)	0	1819	1601	0	3503	1601	1789	3579	1601	1789	3568	
Flt Permitted		0.774			0.817		0.219			0.348		
Satd. Flow (perm)	0	1450	1578	0	2921	1579	412	3579	1563	655	3568	
Satd. Flow (RTOR)			89			165			43		3	
Lane Group Flow (vph)	0	77	55	0	21	14	101	803	24	23	909	
Turn Type	Perm	NA	Perm	Perm	NA	Free	pm+pt	NA	Perm	Perm	NA	
Protected Phases		4			8		5	2			6	
Permitted Phases	4		4	8		Free	2		2	6		
Total Split (s)	32.0	32.0	32.0	32.0	32.0		17.6	50.8	50.8	50.8	50.8	
Total Lost Time (s)		7.0	7.0		7.0		5.6	5.6	5.6	5.6	5.6	
Act Effct Green (s)		9.6	9.6		9.6	62.8	44.2	45.9	45.9	34.9	34.9	
Actuated g/C Ratio		0.15	0.15		0.15	1.00	0.70	0.73	0.73	0.56	0.56	
v/c Ratio		0.35	0.17		0.05	0.01	0.21	0.31	0.02	0.06	0.46	
Control Delay		32.7	4.2		27.6	0.0	5.2	4.8	0.7	12.0	13.5	
Queue Delay		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay		32.7	4.2		27.6	0.0	5.2	4.8	0.7	12.0	13.5	
LOS		С	А		С	А	А	А	А	В	В	
Approach Delay		20.8			16.5			4.7			13.5	
Approach LOS		С			В			А			В	
Queue Length 50th (m)		8.3	0.0		1.1	0.0	3.5	17.8	0.0	1.5	39.8	
Queue Length 95th (m)		23.5	4.6		4.5	0.0	9.0	31.2	1.1	6.0	67.4	
Internal Link Dist (m)		31.8			57.5			328.8			196.7	
Turn Bay Length (m)			40.0			15.0	150.0		85.0	115.0		
Base Capacity (vph)		612	717		1233	1579	569	3314	1451	485	2643	
Starvation Cap Reductn		0	0		0	0	0	0	0	0	0	
Spillback Cap Reductn		0	0		0	0	0	0	0	0	0	
Storage Cap Reductn		0	0		0	0	0	0	0	0	0	
Reduced v/c Ratio		0.13	0.08		0.02	0.01	0.18	0.24	0.02	0.05	0.34	
Intersection Summary												
Cycle Length: 100.4												
Actuated Cycle Length: 62.8												
Control Type: Actuated-Uncoord	dinated											
Maximum v/c Ratio: 0.46												
Intersection Signal Delay: 10.0					tersection							
Intersection Capacity Utilization	63.4%			IC	U Level of	Service E	3					
Analysis Period (min) 15												
Splits and Phases: 3: Hwy 1	& Roberts S	St										
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< 1 ø₂		₽ 04	
50.8 s		32 s	
▲ Ø5	Ø6	↓ Ø8	
17.6 s	50.8 s	32 s	

4: Hwy 1 & N Davis Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	<u>۲</u>	ef 👘			र्स	1	7	^	1	1	^	7
Traffic Volume (vph)	115	34	70	12	39	142	44	596	5	55	695	10
Future Volume (vph)	115	34	70	12	39	142	44	596	5	55	695	10
Satd. Flow (prot)	1610	1524	0	0	1675	1441	1610	3221	1441	1610	3221	144
Flt Permitted	0.409				0.885		0.950			0.950		
Satd. Flow (perm)	689	1524	0	0	1500	1422	1610	3221	1441	1610	3221	144
Satd. Flow (RTOR)		76				198			145			11
Lane Group Flow (vph)	125	113	0	0	55	154	48	648	5	60	755	11
Turn Type	pm+pt	NA		Perm	NA	Free	Prot	NA	Perm	Prot	NA	Perr
Protected Phases	7	4			8		5	2		1	6	
Permitted Phases	4			8		Free			2			
Total Split (s)	15.2	30.4		36.4	36.4		25.6	55.4	55.4	33.5	62.9	62.
Total Lost Time (s)	5.2	5.4			5.4		5.6	6.8	6.8	5.5	6.8	6.
Act Effct Green (s)	20.7	20.5			9.3	74.6	8.5	29.8	29.8	9.1	33.1	33.
Actuated g/C Ratio	0.28	0.27			0.12	1.00	0.11	0.40	0.40	0.12	0.44	0.44
v/c Ratio	0.40	0.24			0.30	0.11	0.26	0.50	0.01	0.31	0.53	0.1
Control Delay	27.7	11.9			40.2	0.2	40.0	20.9	0.0	39.9	19.5	4.
Queue Delay	0.0	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	27.7	11.9			40.2	0.2	40.0	20.9	0.0	39.9	19.5	4.3
LOS	С	В			D	А	D	С	А	D	В	ŀ
Approach Delay		20.2			10.7			22.1			18.9	
Approach LOS		С			В			С			В	
Queue Length 50th (m)	13.9	3.9			7.7	0.0	6.7	39.7	0.0	8.4	47.6	0.
Queue Length 95th (m)	33.5	18.1			21.3	0.0	19.3	62.8	0.0	22.5	73.7	9.0
Internal Link Dist (m)		71.3			45.7			366.0			230.4	
Turn Bay Length (m)							220.0		185.0	250.0		195.
Base Capacity (vph)	334	1024			678	1422	470	2186	1024	658	2436	111
Starvation Cap Reductn	0	0			0	0	0	0	0	0	0	(
Spillback Cap Reductn	0	0			0	0	0	0	0	0	0	
Storage Cap Reductn	0	0			0	0	0	0	0	0	0	(
Reduced v/c Ratio	0.37	0.11			0.08	0.11	0.10	0.30	0.00	0.09	0.31	0.1
Intersection Summary												
Cycle Length: 140.5												
Actuated Cycle Length: 74.6												
Control Type: Actuated-Uncoordi	inated											
Maximum v/c Ratio: 0.53												
ntersection Signal Delay: 19.3				In	tersection l	LOS: B						
Intersection Capacity Utilization 8	54.9%			IC	U Level of	Service A						
Analysis Period (min) 15												

Splits and Phases: 4: Hwy 1 & N Davis Rd

Ø1	Ø2	
33.5 s	55.4 s	30.4 s
▲ ø5	∲ Ø6	Ø7 ▼Ø8
25.6 s	62.9 s	15.2 s 36.4 s

5: Hwy 1 & Davis Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			र्भ	1	1	^	1	1	<u></u>	1
Traffic Volume (veh/h)	13	0	44	3	0	0	13	638	1	5	776	11
Future Volume (Veh/h)	13	0	44	3	0	0	13	638	1	5	776	11
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	14	0	48	3	0	0	14	693	1	5	843	12
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1228	1574	422	1152	1574	346	843			693		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	1228	1574	422	1152	1574	346	843			693		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	89	100	92	98	100	100	98			99		
cM capacity (veh/h)	132	106	581	137	106	650	789			898		
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3	SB 4	
Volume Total	62	3	0	14	346	346	1	5	422	422	12	
Volume Left	14	3	0	14	0	0	0	5	0	0	0	
Volume Right	48	0	0	0	0	0	1	0	0	0	12	
cSH	328	137	1700	789	1700	1700	1700	898	1700	1700	1700	
Volume to Capacity	0.19	0.02	0.00	0.02	0.20	0.20	0.00	0.01	0.25	0.25	0.01	
Queue Length 95th (m)	5.2	0.5	0.0	0.4	0.0	0.0	0.0	0.1	0.0	0.0	0.0	
Control Delay (s)	18.5	31.8	0.0	9.6	0.0	0.0	0.0	9.0	0.0	0.0	0.0	
Lane LOS	С	D	А	А				А				
Approach Delay (s)	18.5	31.8		0.2				0.1				
Approach LOS	С	D										
Intersection Summary												
Average Delay			0.9									
Intersection Capacity Utilization			41.0%	IC	U Level of	Service			А			
Analysis Period (min)			15									

6: Hwy 1 & Thicke Rd/Edgelow Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		- N	≜t ≽		- h	*	7
Traffic Volume (vph)	21	0	1	4	0	4	4	622	3	4	819	6
Future Volume (vph)	21	0	1	4	0	4	4	622	3	4	819	6
Satd. Flow (prot)	0	1786	0	0	1713	0	1789	3575	0	1789	3579	1601
Flt Permitted							0.320			0.393		
Satd. Flow (perm)	0	1872	0	0	1755	0	603	3575	0	740	3579	1601
Satd. Flow (RTOR)		47			47			1				35
Lane Group Flow (vph)	0	24	0	0	8	0	4	679	0	4	890	7
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	Perm
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		6
Total Split (s)	25.7	25.7		25.7	25.7		57.4	57.4		58.1	58.1	58.1
Total Lost Time (s)		5.7			5.7		6.6	6.6		6.6	6.6	6.6
Act Effct Green (s)		7.0			7.0		39.7	39.7		39.7	39.7	39.7
Actuated g/C Ratio		0.16			0.16		0.92	0.92		0.92	0.92	0.92
v/c Ratio		0.07			0.02		0.01	0.21		0.01	0.27	0.00
Control Delay		4.1			0.1		2.5	1.7		2.5	1.9	0.2
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	0.0
Total Delay		4.1			0.1		2.5	1.7		2.5	1.9	0.2
LOS		А			А		А	А		А	А	A
Approach Delay		4.1			0.1			1.7			1.9	
Approach LOS		А			А			А			А	
Queue Length 50th (m)		0.0			0.0		0.0	0.0		0.0	0.0	0.0
Queue Length 95th (m)		2.9			0.0		0.9	19.1		0.9	26.3	0.2
Internal Link Dist (m)		76.7			10.6			788.3			1015.2	
Turn Bay Length (m)							160.0			175.0		175.0
Base Capacity (vph)		893			839		603	3575		740	3579	1601
Starvation Cap Reductn		0			0		0	0		0	0	0
Spillback Cap Reductn		0			0		0	0		0	0	0
Storage Cap Reductn		0			0		0	0		0	0	0
Reduced v/c Ratio		0.03			0.01		0.01	0.19		0.01	0.25	0.00
Intersection Summary												
Cycle Length: 83.8												
Actuated Cycle Length: 43.2												
Control Type: Actuated-Uncoordin	nated											
Maximum v/c Ratio: 0.27												
Intersection Signal Delay: 1.8				In	tersection I	LOS: A						
Intersection Capacity Utilization 5	4.9%			IC	U Level of	Service A						
Analysis Period (min) 15												

Splits and Phases: 6: Hwy 1 & Thicke Rd/Edgelow Rd

	<u>→</u> _{Ø4}	
57.4 s	25.7 s	
	₩ Ø8	
58.1 s	25.7 s	

1: Hwy 1 & Grouhel Rd

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Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	¥		ሻ	^	^	1			
raffic Volume (veh/h)	31	19	26	1489	1416	48			
future Volume (Veh/h)	31	19	26	1489	1416	48			
Sign Control	Stop	10	20	Free	Free	10			
rade	0%			0%	0%				
eak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92			
ourly flow rate (vph)	34	21	28	1618	1539	52			
edestrians	J .	21	20	1010	1000	52			
ane Width (m)									
alking Speed (m/s)									
ercent Blockage									
tight turn flare (veh)									
ledian type				None	None				
ledian storage veh)				NULLE	NULLE				
lpstream signal (m)									
X, platoon unblocked									
C, conflicting volume	2404	770	1539						
C1, stage 1 conf vol	2404	770	1009						
C2, stage 2 conf vol									
Cu, unblocked vol	2404	770	1539						
Cu, unblocked voi	6.8	6.9	4.1						
	0.0	0.9	4.1						
C, 2 stage (s)	3.5	3.3	2.2						
(s)									
0 queue free %	0	94	93						
M capacity (veh/h)	26	344	428						
irection, Lane #	EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	SB 3		
olume Total	55	28	809	809	770	770	52		
'olume Left	34	28	0	0	0	0	0		
olume Right	21	0	0	0	0	0	52		
SH	40	428	1700	1700	1700	1700	1700		
olume to Capacity	1.38	0.07	0.48	0.48	0.45	0.45	0.03		
ueue Length 95th (m)	42.4	1.6	0.0	0.0	0.0	0.0	0.0		
control Delay (s)	432.5	14.0	0.0	0.0	0.0	0.0	0.0		
ane LOS	F	В							
pproach Delay (s)	432.5	0.2			0.0				
pproach LOS	F								
tersection Summary									
erage Delay			7.3						
tersection Capacity Utilization			51.2%	IC	U Level of	Service		A	
nalysis Period (min)			15					,.	
			10						

2: Hwy 1 & 1st Ave/Ludlow Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	f,		ሻ	↑	1	۳.	*	1	ሻ	^	1
Traffic Volume (vph)	255	36	32	70	79	23	17	1226	48	38	1101	242
Future Volume (vph)	255	36	32	70	79	23	17	1226	48	38	1101	242
Satd. Flow (prot)	1789	1739	0	1789	1883	1601	1789	3579	1601	1789	3579	1601
Flt Permitted	0.701			0.709			0.235			0.100		
Satd. Flow (perm)	1320	1739	0	1334	1883	1601	443	3579	1565	188	3579	1601
Satd. Flow (RTOR)		35				185			112			263
Lane Group Flow (vph)	277	74	0	76	86	25	18	1333	52	41	1197	263
Turn Type	Perm	NA		Perm	NA	Free	Perm	NA	Perm	pm+pt	NA	Perm
Protected Phases		4			8			2		1	6	
Permitted Phases	4			8		Free	2		2	6		6
Total Split (s)	22.0	22.0		24.2	24.2		51.4	51.4	51.4	20.7	52.1	52.1
Total Lost Time (s)	7.0	7.0		7.2	7.2		6.4	6.4	6.4	5.7	6.4	6.4
Act Effct Green (s)	15.4	15.4		15.2	15.2	76.0	40.3	40.3	40.3	47.5	46.8	46.8
Actuated g/C Ratio	0.20	0.20		0.20	0.20	1.00	0.53	0.53	0.53	0.62	0.62	0.62
v/c Ratio	1.04	0.19		0.29	0.23	0.02	0.08	0.70	0.06	0.16	0.54	0.24
Control Delay	101.9	19.8		33.5	31.6	0.0	11.2	16.4	0.1	6.1	8.9	1.3
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	101.9	19.8		33.5	31.6	0.0	11.2	16.4	0.1	6.1	8.9	1.3
LOS	F	В		С	С	А	В	В	А	А	А	A
Approach Delay		84.6			28.1			15.7			7.5	
Approach LOS		F			С			В			А	
Queue Length 50th (m)	~55.2	5.4		11.0	12.3	0.0	1.4	80.7	0.0	1.9	44.7	0.0
Queue Length 95th (m)	#102.0	16.8		23.6	25.3	0.0	4.8	106.6	0.3	4.8	57.8	6.7
Internal Link Dist (m)		181.2			128.9			325.9			168.6	
Turn Bay Length (m)	90.0			65.0			140.0		130.0	160.0		
Base Capacity (vph)	307	431		306	432	1601	269	2177	996	442	3078	1413
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.90	0.17		0.25	0.20	0.02	0.07	0.61	0.05	0.09	0.39	0.19
Intersection Summary												
Cycle Length: 96.3												
Actuated Cycle Length: 76												
Control Type: Actuated-Unco	ordinated											
Maximum v/c Ratio: 1.04												
Intersection Signal Delay: 19.					tersection l							
Intersection Capacity Utilizati	on 65.9%			IC	U Level of	Service C						
Analysis Period (min) 15												
 Volume exceeds capacity 			nfinite.									
Queue shown is maximum												
# 95th percentile volume ex			nay be long	ger.								
Queue shown is maximun	n after two cyc	es.										
Splits and Phases: 2: Hwy	1 & 1st Ave/Li	udlow Rd										
	€								<u>_</u>			
Ø1 20.7 s	51.4 s								- Ø4	-		
20.75	51.45								22.5			

♥ Ø6

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24.2 s

3: Hwy 1 & Roberts St

E EBT 3 15 3 15 3 15 0 1808 0.711 0 1322 0 95 m NA 4	EBR 93 93 1601 1565 101 101	WBL 39 39 0	WBT 26 26 3475 0.764	WBR 7 39 39 1601	NBL 142 142	NBT 1236 1236	NBR 7 39 39	SBL ř 29	SBT †1	SBF
3 15 3 15 0 1808 0.711 0 0 1322 0 95 m NA	93 93 1601 1565 101	39 0	26 26 3475	39 39	142 142	1236	39	29		
3 15 0 1808 0.711 0 0 1322 0 95 m NA	93 1601 1565 101	39 0	26 3475	39	142				1195	
0 1808 0.711 0 1322 0 95 m NA	1601 1565 101	0	3475			1236	20			4
0.711 0 1322 0 95 m NA	1565 101			1601			39	29	1195	4
0 1322 0 95 m NA	101	0	0 764	1001	1789	3579	1601	1789	3557	
0 95 m NA	101	0	0.10-		0.094			0.203		
m NA			2719	1576	177	3579	1541	381	3557	
m NA	101			165			43		4	
		0	70	42	154	1343	42	32	1344	
4	Perm	Perm	NA	Free	pm+pt	NA	Perm	Perm	NA	
			8		5	2			6	
4	4	8		Free	2		2	6		
.0 32.0	32.0	32.0	32.0		17.6	50.8	50.8	50.8	50.8	
7.0	7.0		7.0		5.6	5.6	5.6	5.6	5.6	
11.4	11.4		11.4	81.3	57.1	57.1	57.1	41.7	41.7	
0.14	0.14		0.14	1.00	0.70	0.70	0.70	0.51	0.51	
0.52	0.33		0.18	0.03	0.49	0.53	0.04	0.16	0.74	
44.5	10.5		33.5	0.0	13.8	7.0	1.6	14.6	19.1	
0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
44.5	10.5		33.5	0.0	13.8	7.0	1.6	14.6	19.1	
D	В		С	А	В	А	А	В	В	
27.0			20.9			7.5			19.0	
С			С						В	
14.5	0.0		5.3	0.0	6.4	42.7	0.0	2.5	79.2	
29.8	12.9		11.3	0.0	24.1	71.2	2.8	8.9	125.0	
31.8			57.5			328.8			196.7	
	40.0			15.0	150.0		85.0	115.0		
413	558		850	1576	366	2811	1219	215	2013	
0	0		0	0	0	0	0	0	0	
0	0		0	0	0	0	0	0	0	
0	0		0	0	0	0	0	0	0	
0.23	0.18		0.08	0.03	0.42	0.48	0.03	0.15	0.67	
		Ini	arcaction	00. D						
				-05: B						
			U Level of)					
	44.5 D 27.0 C 14.5 29.8 31.8 413 0 0 0	44.5 10.5 D B 27.0 C 14.5 0.0 29.8 12.9 31.8 40.0 413 558 0 0 0 0 0 0 0 0 0 0 0 0	44.5 10.5 D B 27.0 C 14.5 0.0 29.8 12.9 31.8 40.0 413 558 0 0 0 0 0 0 0.23 0.18	$\begin{array}{c cccccc} 44.5 & 10.5 & 33.5 \\ D & B & C \\ 27.0 & 20.9 \\ C & C \\ 14.5 & 0.0 & 5.3 \\ 29.8 & 12.9 & 11.3 \\ 31.8 & 57.5 \\ & 40.0 \\ 413 & 558 & 850 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 &$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$

Splits and Phases: 3: Hwy 1 & Roberts St

√ ø2		₽ 04	
50.8 s		32 s	
▲ ø5		₩ Ø8	
17.6 s	50.8 s	32 s	

4: Hwy 1 & N Davis Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations	ሻ	1.			र्स	1	ሻ	*	1	<u>۲</u>	- † †	7
Traffic Volume (vph)	173	51	104	11	66	156	128	1122	23	183	928	235
Future Volume (vph)	173	51	104	11	66	156	128	1122	23	183	928	235
Satd. Flow (prot)	1610	1524	0	0	1683	1441	1610	3221	1441	1610	3221	1441
Flt Permitted	0.456				0.922		0.950			0.950		
Satd. Flow (perm)	767	1524	0	0	1563	1421	1610	3221	1407	1609	3221	1441
Satd. Flow (RTOR)		78				198			145			255
Lane Group Flow (vph)	188	168	0	0	84	170	139	1220	25	199	1009	255
Turn Type	pm+pt	NA		Perm	NA	Free	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	7	4			8		5	2		1	6	
Permitted Phases	4			8		Free			2			6
Total Split (s)	15.2	30.4		36.4	36.4		25.6	55.4	55.4	33.5	62.9	62.9
Total Lost Time (s)	5.2	5.4			5.4		5.6	6.8	6.8	5.5	6.8	6.8
Act Effct Green (s)	23.8	23.6			11.3	110.6	14.7	49.9	49.9	19.1	54.2	54.2
Actuated g/C Ratio	0.22	0.21			0.10	1.00	0.13	0.45	0.45	0.17	0.49	0.49
v/c Ratio	0.78	0.43			0.53	0.12	0.65	0.84	0.04	0.72	0.64	0.31
Control Delay	61.5	24.0			61.8	0.2	62.0	35.6	0.1	59.2	25.1	3.5
Queue Delay	0.0	0.0			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	61.5	24.0			61.8	0.2	62.0	35.6	0.1	59.2	25.1	3.5
LOS	E	С			E	А	E	D	А	E	С	A
Approach Delay		43.8			20.5			37.6			26.0	
Approach LOS		D			С			D			С	
Queue Length 50th (m)	35.8	16.1			17.8	0.0	29.2	126.0	0.0	41.7	87.5	0.0
Queue Length 95th (m)	#67.2	38.0			35.9	0.0	53.3	#202.9	0.0	68.9	127.7	14.5
Internal Link Dist (m)		71.3			45.7			366.0			230.4	
Turn Bay Length (m)							220.0		185.0	250.0		195.0
Base Capacity (vph)	242	691			444	1421	295	1452	714	414	1670	870
Starvation Cap Reductn	0	0			0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0			0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0			0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.78	0.24			0.19	0.12	0.47	0.84	0.04	0.48	0.60	0.29
Intersection Summary												
Cycle Length: 140.5												
Actuated Cycle Length: 110.6												
Control Type: Actuated-Uncoord	linated											
Maximum v/c Ratio: 0.84												
Intersection Signal Delay: 32.1												
Intersection Capacity Utilization	77.8%			IC	U Level of	Service D						
Analysis Period (min) 15												
# 95th percentile volume excee			nay be long	ger.								
Queue shown is maximum af	ter two cycl	es.										

Splits and Phases: 4: Hwy 1 & N Davis Rd

Ø1		Ø2	 ∕⊉ø₄		
33.5 s		55.4 s	30.4 s		
▲ ø5	4 ø	6	▶ Ø7	₹ø8	
25.6 s	62.9 s		15.2 s	36.4 s	

5: Hwy 1 & Davis Rd

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			÷٩	1	7	^	1	7	^	1
Traffic Volume (veh/h)	10	0	41	0	1	1	88	1235	2	10	974	53
Future Volume (Veh/h)	10	0	41	0	1	1	88	1235	2	10	974	53
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	0	45	0	1	1	96	1342	2	11	1059	58
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage veh)								Homo			Tiono	
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	1944	2615	530	2086	2615	671	1059			1342		
vC1, stage 1 conf vol	1011	2010	000	2000	2010	011	1000			1012		
vC2, stage 2 conf vol												
vCu, unblocked vol	1944	2615	530	2086	2615	671	1059			1342		
tC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1			4.1		
tC, 2 stage (s)	1.0	0.0	0.0	1.0	0.0	0.0	1.1			1.1		
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	66	100	91	100	95	100	85			98		
cM capacity (veh/h)	33	20	494	24	20	399	653			509		
Direction, Lane #	EB 1	WB 1	WB 2	NB 1	NB 2	NB 3	NB 4	SB 1	SB 2	SB 3	SB 4	
Volume Total	56	1	1	96	671	671	2	11	530	530	58	
Volume Left	11	0	0	96	0	0	0	11	0	0	0	
Volume Right	45	0	1	0	0	0	2	0	0	0	58	
cSH	131	20	399	653	1700	1700	1700	509	1700	1700	1700	
Volume to Capacity	0.43	0.05	0.00	0.15	0.39	0.39	0.00	0.02	0.31	0.31	0.03	
Queue Length 95th (m)	14.2	1.1	0.1	3.9	0.0	0.0	0.0	0.5	0.0	0.0	0.0	
Control Delay (s)	51.7	194.8	14.0	11.5	0.0	0.0	0.0	12.2	0.0	0.0	0.0	
Lane LOS	F	F	В	В				В				
Approach Delay (s)	51.7	104.4		0.8				0.1				
Approach LOS	F	F										
Intersection Summary												
Average Delay			1.7									
Intersection Capacity Utilization			61.4%	IC	U Level of	Service			В			
Analysis Period (min)			15									

6: Hwy 1 & Thicke Rd/Edgelow Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Lane Configurations		4			4		7	A		<u>م</u>	^	1
Traffic Volume (vph)	53	1	2	1	1	4	17	1260	3	3	975	13
Future Volume (vph)	53	1	2	1	1	4	17	1260	3	3	975	13
Satd. Flow (prot)	0	1791	0	0	1700	0	1789	3579	0	1789	3579	1601
Flt Permitted		0.765			0.928		0.263			0.175		
Satd. Flow (perm)	0	1435	0	0	1591	0	495	3579	0	330	3579	1601
Satd. Flow (RTOR)		2			4							3
Lane Group Flow (vph)	0	61	0	0	6	0	18	1373	0	3	1060	14
Turn Type	Perm	NA		Perm	NA		Perm	NA		Perm	NA	Perm
Protected Phases		4			8			2			6	
Permitted Phases	4			8			2			6		6
Total Split (s)	25.7	25.7		25.7	25.7		57.4	57.4		58.1	58.1	58.1
Total Lost Time (s)		5.7			5.7		6.6	6.6		6.6	6.6	6.6
Act Effct Green (s)		8.9			8.9		48.0	48.0		48.0	48.0	48.0
Actuated g/C Ratio		0.15			0.15		0.81	0.81		0.81	0.81	0.81
v/c Ratio		0.28			0.02		0.04	0.47		0.01	0.37	0.01
Control Delay		29.4			20.0		4.2	4.9		4.0	4.1	0.5
Queue Delay		0.0			0.0		0.0	0.0		0.0	0.0	0.0
Total Delay		29.4			20.0		4.2	4.9		4.0	4.1	0.5
LOS		С			В		А	А		А	А	A
Approach Delay		29.4			20.0			4.9			4.1	
Approach LOS		С			В			А			А	
Queue Length 50th (m)		6.8			0.2		0.6	36.3		0.1	24.6	0.0
Queue Length 95th (m)		17.7			3.3		2.6	59.3		0.9	40.5	0.6
Internal Link Dist (m)		76.7			10.6			788.3			1015.2	
Turn Bay Length (m)							160.0			175.0		175.0
Base Capacity (vph)		510			567		418	3025		278	3025	1358
Starvation Cap Reductn		0			0		0	0		0	0	C
Spillback Cap Reductn		0			0		0	0		0	0	(
Storage Cap Reductn		0			0		0	0		0	0	(
Reduced v/c Ratio		0.12			0.01		0.04	0.45		0.01	0.35	0.01
Intersection Summary												
Cycle Length: 83.8												
Actuated Cycle Length: 59.4												
Control Type: Actuated-Uncoordina	ated											
Maximum v/c Ratio: 0.47												
Intersection Signal Delay: 5.2	5.2 Intersection LOS: A											
Intersection Capacity Utilization 55	.0%			IC	U Level of	Service A						
Analysis Period (min) 15												

Splits and Phases: 6: Hwy 1 & Thicke Rd/Edgelow Rd

≪¶ ø2	
57.4 s	25.7 s
	€ Ø8
58.1 s	25.7 s